ADDENDUM NO. 3

July 13, 2026

SCHOOL CITY OF HOBART VARSITY BASEBALL AND SOFTBALL TURF FIELDS PROJECT

TO: ALL BIDDERS OF RECORD

This Addendum forms a part of and modifies the Bidding Requirements, Contract Forms, Contract Conditions, the Specifications, and the Drawings dated June 29, 2026 by Gibraltar Design. Acknowledge receipt of the Addendum in the space provided on the Bid Form. Failure to do so may subject the Bidder to disqualification.

This Addendum consists of page ADD 3-1 dated July 13, 2026, consisting of 1 page.

A. SPECIFICATION SECTION 00 20 00 - INFORMATION AVAILABLE TO BIDDERS

1. **Add:**

   The attached Geotechnical Engineering Report.

B. DRAWINGS

1. **Sheet E-601**

   Remove the phrase “Alternate Bid” from the Athletic Field Relay Schedule Title.

July 9, 2026

Ms. Veronica Kessler
President
Construction Field Services, LLC
8762 Louisiana Street, Suite N
Merrillville, IN 46410

Subject: Geotechnical Engineering Report
Hobart High School Softball & Baseball Fields
2211 E 10th Street
Hobart, IN 46342
GEOCON Project No. 26-G0856

Dear Ms. Kessler:

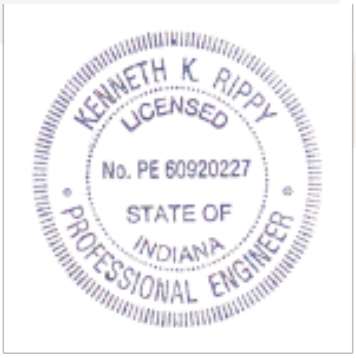
Pursuant to our proposal for geotechnical engineering services, we have completed subsurface exploration and geotechnical analyses for the above-referenced project. This Geotechnical Engineering Report includes our findings and recommendations for the proposed turf field project.

GEOCON Professional Services, Inc. (GEOCON) appreciates the opportunity to be of service during this phase of the project. If there are any questions or comments you may have regarding the contents of this report, or if we may be of any further service, please contact us at your convenience.

Sincerely,

GEOCON Professional Services, LLC.

Brandon Filafusi, EIT
Project Engineer



Kenneth K. Rippy, PE
Senior Geotechnical Engineer

GEOTECHNICAL ENGINEERING REPORT

Hobart High School Softball & Baseball Fields
2211 E 10th Street
Hobart, IN 46342

Ms. Veronica Kessler
President
Construction Field Services, LLC
8762 Louisiana Street, Suite N
Merrillville, IN 46410

Prepared by:
GEOCON Professional Services, LLC.
22774 Citation Road, Unit A
Frankfort, Illinois 60423

July 9, 2026

GEOCON Project No. 26-G0856

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GEOTECHNICAL ENGINEERING REPORT
Hobart High School Softball & Baseball Fields
2211 E 10th Street
Hobart, IN 46342

INTRODUCTION

This report presents the results of a subsurface exploration for the proposed site at the existing Hobart High School baseball and softball fields referenced above. The purpose of this report was to determine and evaluate the subsurface conditions existing at the subject site, and to establish related geotechnical parameters to be utilized for the economical design and construction for this project.

Authorization to perform this subsurface exploration and analysis was provided in the form of a signed Consultant Agreement referencing GEOCON Proposal No 26-P588 REVISED, dated May 26, 2026. The above-mentioned document described the project scope and contained general conditions for performance of the work.

SITE AND PROJECT DESCRIPTION

The proposed project is located at the south end of Hobart High School on the existing baseball and softball fields. Historical aerial and topographic maps available online at http://www.historicaerials.com were reviewed for the subject site. Based on our review of the aerial maps, it appears that this site has no previous developments since the construction of the existing school and fields. Current site use in the proposed improvement area consists of grass landscape areas and concrete walking paths.

The project will consist of constructing new synthetic turf fields and bullpens within the existing natural grass baseball and softball fields. It is assumed that the finished grade of the new turf areas will be near existing grades of about El. 632 feet to El. 635 feet. Additional sitework will include new exterior concrete walkways.

SUBSURFACE EXPLORATION

The scope of the exploration, including the number, location, and depth of the borings, was determined by the client. A total of eight (8) borings were advanced for the project at the approximate locations shown on the Boring Location Diagram included in the Appendix. Three (3) borings were performed at the north-east baseball field, three (3) at the south-west softball field, and two (2) on the concrete pathways surrounding each respective field each extending to a total termination depth of 10 feet.

The boring locations were marked in the field by Construction Field Services (CFS) prior to drilling and surface elevations noted on the logs were estimated from Google Earth. After completion of the borings, the holes were backfilled with soil cuttings and patched with like materials as encountered at the boring. Additionally, the placement of seed and topsoil at each proposed boring was performed upon completion by a GEOCON representative.

For safety reasons, the boreholes were not allowed to remain open and precluded the collection of delayed water readings.

Drilling and Sampling Procedures

The soil borings were performed with an ATV-mounted drilling rig equipped with a rotary head. Conventional, continuous flight, hollow-stem augers were used to advance the borings with representative samples obtained in each boring employing split-barrel sampling techniques in accordance with ASTM Procedure D-1586. Soil samples were secured at 2.5-foot intervals to boring termination depth.

The Standard Penetration Test (SPT) is defined as the number of blows required to advance a 2-inch O.D., split-barrel sampler a distance of one foot by a 140-pound hammer falling 30 inches, commonly described as the N-value. These sampler resistances provide a useful indication of the consistency or relative density of most soil deposits and are reported on the boring logs presented in the Appendix. Samples of cohesive soils obtained from the borings were tested with a calibrated hand penetrometer to aid in evaluating the soil strength characteristics. The results from this testing is tabulated on the boring logs.

Water level observations were made during drilling operations and upon completion and of the borings. This data is noted on the boring logs and within the *Groundwater Conditions* section.

It should be noted that it is difficult to determine the stratigraphy of the upper 2 to 3 feet of the profile from soil borings due to the size of the bore hole, about 6 inches in diameter, and intermittent sample intervals. Further, the split spoon sampler tends to push through softer soils such as fill or topsoil, resulting in little or no sample recovery from these soils. It is recommended that shallow test pits be excavated to better define the exact depth of topsoil or fill if such information is required prior to construction.

Laboratory Tests

Additional characteristics of the foundation materials were determined in the laboratory to provide data on which to classify and estimate the engineering properties of the subsurface soil deposits encountered in the borings. All samples were visually classified by the geotechnical engineer according to the Unified Soil Classification System (ASTM D-2488). An explanation of the symbols used in this system is included in the Appendix.

Representative samples were tested in the laboratory to determine the natural moisture content of the soils. All moisture contents are expressed as a percentage of the dry weight of soil. Representative samples of the cohesive soils encountered in the borings were tested in the laboratory with a calibrated RIMAC spring tester to determine the approximate unconfined compressive strength of the soil samples.

The laboratory testing program selected for this project is intended to assist with determination of soil classification as well as strength and deformation characteristics of the subsurface soil deposits that will provide foundation support for the proposed structures. All laboratory testing was performed in general accordance with the respective ASTM Methods, as applicable, and the results are included on the boring logs included in the Appendix. Unless notified to the contrary, all samples will be disposed of after one month.

SOIL CONDITIONS

The types of materials encountered at the test boring locations are described on the Soil Boring Logs. The lines delineating the changes in strata on the logs represent an approximate boundary between the various soil classifications. It must be recognized that the soil descriptions are considered representative for the specific test hole location, but the variations may occur between the sampling intervals and boring locations. A summary of the major soil profile components is described in the following paragraphs. A more detailed description and supporting data for each boring location can be found on the individual boring logs.

Surface conditions varied at the boring locations. Borings B-1 and B-5 were advanced through 4 to 6 inches of concrete underlain by 0 to 2 inches of aggregate base. Borings B-3, B-4, B-6, B-7, and B-8 were performed on the grass landscape within or around the baseball and softball fields, where 4 to 6 inches

of topsoil was encountered. Boring B-2, located in the softball infield, encountered approximately 4 to 6 inches of sand.

The soil profile underlying the surficial concrete and topsoil consisted primarily of brown and gray native sandy, silty, lean clay generally extending to depths of approximately 3.5 to 10 feet below grade. The clay soils were described as medium stiff to very stiff with unconfined compressive strengths ranging from 0.5 to 2.5 tsf except for a very soft layer at B-4 from 6 to 7.5 feet below grade. Moisture contents of samples of the clay ranged from 11.2 to 28.7 percent.

The clay soils were interstratified by brown and gray silt and sand soils encountered between depths of 1 to 10 feet below grade. These noncohesive soils were classified as loose to medium dense with ranging N-values of 5 to 18 blows per foot (bpf) and moisture contents ranging between 12.6 to 27.3 percent. It is important to note that within the sand and silt soils, they were generally observed to be wet, indicating potential groundwater.

Further information regarding the soil conditions can be found on the boring logs included in the Appendix.

GROUNDWATER CONDITIONS

All borings except for B-2 encountered groundwater at the time of drilling at depths of 3.5 to 8.5 feet below ground surface, which correlates with the poorly graded sand and silt layers. It should be noted that due to the relatively limited stabilization times under which the water levels measurements were made, these observed groundwater level readings may not indicate the static water level for the site at the time of drilling. In addition, groundwater levels fluctuate over time and are influenced by seasonal precipitation and varying permeability characteristics of the subsurface soils.

ENGINEERING RECOMMENDATIONS

Site Preparation and Earthwork

After stripping the surface in structure and sidewalk areas, the subgrade soils be proof rolled or tested with a DCP to identify any weak or unsuitable areas at or just below the subgrade elevation prior to placement of grade raised fill. Proof rolling may be accomplished with a fully loaded single axle dump truck or other pneumatic tire equipment which provides a similar subgrade loading.

Areas that experience rutting or pumping under the proof roll load should be improved by subgrade improvement methods such as disking and drying, or removal and replacement. Disking, drying and recompaction must be carried out during suitable weather conditions which allows for drying of the subgrade prior to recompaction. The removal and replacement method involves undercutting the excessively wet or soft soils and replacing them with 3-inch crushed aggregate or equivalent. The aggregate can be supplemented with geogrid, placed, and compacted in minimum lifts of 1 foot and capped off with a minimum of 6 inches of INDOT #53's aggregate or equivalent to reduce surface water to infiltrate the subgrade. After the subgrade soils are stabilized, and suitable for support of new site grading fill or the floor slab, low areas may then be raised to the planned grades with properly compacted fill as described in the following section.

The existing subgrade soils will be susceptible to disturbance from precipitation, construction traffic, and vibrations. Care should be taken to avoid disturbance of the subgrade soil and construction traffic over prepared subgrades should be avoided. If the subgrade soils become disturbed during construction, they should be scarified and recompacted or removed and replaced prior to placing new site grading fill or granular subbase material.

Mass grading and earthwork operations should be observed and evaluated by a representative of the geotechnical engineer.

Controlled Compacted Fill

All new fill and aggregate materials should be placed on firm subgrades, and the fill should be placed in lifts and properly compacted. All newly placed fill should be placed in 9 inch or less loose lifts and compacted to at least 95 percent of the maximum dry density as determined by ASTM D-1557 (Modified Proctor) method of test. The fill should be placed within +/- 2 percent of the optimum moisture content value determined by laboratory Proctor testing.

The site should be graded to promote runoff of surface water in order to minimize ponding of precipitation on the prepared subgrades, or in excavations. If the subgrade becomes saturated, or becomes deteriorated from repeated construction traffic, the affected material should be removed, and these materials should be disked and recompacted or undercut and replaced with suitable fill prior to further construction in those areas.

GEOCON recommends that the evaluation of the subgrade and selection of fill materials for various applications should be done in consultation with the geotechnical engineer, and placement of fill for structural applications be monitored and tested by a representative of the geotechnical engineer.

Spread Footings

Based on the soil conditions encountered during the subsurface investigation and laboratory test results, spread footings may be used to support the proposed bleacher grandstands and back stop walls. Spread footing foundations may be placed directly on suitable native clay soil provided the unconfined compressive strength is 0.75 tsf or greater, or on native sand or silt exhibiting a correlated N-value of 6 blows per foot (bpf) or greater. Spread footings can be designed for a net allowable bearing pressure of 1,500 pounds per square foot (psf). The net allowable soil bearing pressure is the pressure in excess of the minimum surrounding overburden pressure at the footing base elevation.

Field observation of the foundation subgrade soil should be performed by a representative of the geotechnical engineer at the time of construction. At a minimum, the foundation observation and testing program should include visual inspection of the foundation excavations prior to concrete placement, to verify material type and confirm that any loose or otherwise disturbed soil has been removed from the excavation. Dynamic Cone Penetrometer (DCP) and hand penetrometer testing should be performed to verify that the soils present within the influence zone of the foundations exhibit a minimum Q_u of 0.75 tsf or correlated N-value of 6 bpf.

Unsuitable or lower strength bearing soils present within the influence zone of the foundation systems that do not meet the minimum strength requirements described above should be removed and replaced with granular material consisting of 3-inch crushed limestone aggregate capped off with a minimum of 6 inches of INDOT 53s or equivalent. Fill materials placed below the foundations should extend a minimum of 6 inches beyond the edge of the footings for each foot of fill placed. Lean concrete does not appear to be a viable option for backfilling undercuts due to the cohesionless soil encountered. **If saturated sand or silt soil layers are encountered during foundation excavation, the bearing soil may become disturbed from excess hydrostatic pressure after removal of the overburden soil. If groundwater seepage occurs during construction of the foundations, dewatering will be required to maintain groundwater levels below the depth of excavation. See the Groundwater Control section for further information.**

It is generally recommended that exterior footings be placed at a minimum depth of 3.5 feet below the outside finished grade for frost protection purposes. All footings should be protected from the effects of frost if construction is carried out during winter months.

CONSTRUCTION CONSIDERATIONS

Groundwater Control

As previously stated, borehole groundwater was encountered during the drilling operations within sand or silt strata. The boreholes were backfilled immediately after completion of drilling and delayed groundwater measurements were not collected at this site. Based on the possibility of encountering perched water or water bearing sand or silt seams interbedded within the predominantly clay subgrade, the contractor should be prepared to provide adequate means of dewatering. Dewatering system design is the responsibility of the excavation contractor or an experienced dewater subcontractor. **Note that sand and silt soils are difficult to dewater, often requiring extensive dewatering efforts such as installation of a well point dewatering system and operation of the dewatering system over an extended period.** All sumps should be properly filtered to avoid pumping soil fines.

When designing site drainage patterns, site runoff should be diverted away from the foundations and directed towards on-site detention areas, or storm sewer systems. Such measures reduce the potential for softening and possible erosion of the foundation and pavement subgrade soils. It is especially important that water not be allowed to collect next to the building foundations.

Excavations

All excavations should comply with the requirements of OSHA 29CFR, Part 1926, Subpart P, "Excavations," regarding excavation and trench safety, as well as other applicable codes. This document states that excavation safety is the sole responsibility of the contractor and accordingly reference to this OSHA requirement should be included in the project specifications. Excavation slopes shall in no case be steeper than those specified by OSHA, and all excavations should be monitored by a competent person, as defined by the OSHA standard. Appropriate shoring or sloping techniques should be used to prevent cave-ins.

Excavations near existing foundations, pavements or utilities should be made with caution as disturbance within foundation influence zones that support adjacent structures could result in excessive settlement. If the proposed construction extends into the influence zone of the existing foundations, a shoring or underpinning system may be required to protect or support the adjacent structures.

GENERAL COMMENTS

This geotechnical exploration and foundation analysis has been conducted to aid in the evaluation of the foundation conditions on the subject site. The recommendations presented herein are based on the available soil information obtained and the design information provided. Any changes in the soil conditions encountered during construction, design, or building locations should be brought to the attention of the soils engineer to determine if modifications in the recommendations are required. The final design plans and specifications should also be reviewed by the soils engineer to determine that the recommendations presented herein have been interpreted and implemented as intended. It is recommended that the earthwork and foundation operations be monitored by the Geotechnical Engineer, to test and evaluate the bearing capacities, and the selection, placement and compaction of controlled fills.

This geotechnical study has been conducted in a manner consistent with that level of care ordinarily exercised by members of the profession currently practicing in the same locality under similar

conditions. The findings, recommendations, and opinions contained herein have been promulgated in accordance with generally accepted practice in the fields of foundation engineering, soils mechanics, and engineering geology. No other representations, expressed or implied, and no warranty or guarantee is included or intended in this report.

APPENDIX

Boring Location Diagram
Soil Boring Logs
General Notes

Hobart High School

2211 E 10th St, Hobart, IN 46342

Legend

B-1

B-2

B-3

B-4

B-5

B-6

B-7

B-8

HOBART

BRONCOS

Google Earth

Image © 2026 Vexcel Imaging US, Inc.

N

400 ft

GEOCON PROFESSIONAL SERVICES

BORING NO. B-1

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	631.5		6" CONCRETE											
			brown and gray SANDY CLAY medium stiff	SS 1	56	7-6-7 (13)	0.5		11.2					
2.5	628.5		brown and gray SILT medium dense	SS 2	78	4-5-5 (10)			14.4					
5.0			wet at 6 ft	SS 3	89	5-5-9 (14)			21.0					
7.5	623.5		brown and gray LEAN CLAY silt seam at 9 ft stiff	SS 4	100	5-5-3 (8)	1.0	1.1	20.3					
10.0	622.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 632 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING 6.00 ft / Elev 626.00 ft

AT END OF DRILLING 6.00 ft / Elev 626.00 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

GEOCON PROFESSIONAL SERVICES

BORING NO. B-2

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	632.5		4" - 6" SAND											
			dark brown and gray SILT medium dense	SS 1	78	4-6-5 (11)			14.5					
2.5	629.5		brown and gray LEAN CLAY stiff	SS 2	89	3-3-3 (6)	1.0		28.7					
5.0	627.0		brown and gray SILTY CLAY stiff	SS 3	56	3-3-2 (5)	1.5		28.0			24	20	4
7.5	624.5		brown and gray LEAN CLAY medium stiff	SS 4	78	8-6-6 (12)	0.75	0.8	20.1					
10.0	623.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 633 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING --- None

AT END OF DRILLING --- Dry upon completion

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

BORING NO. B-3

GEOCON PROFESSIONAL SERVICES

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	629.5		4" - 6" TOPSOIL											
			brown SILTY SAND loose	SS 1	56	5-4-3 (7)			16.8					
2.5			wet at 3.5 ft	SS 2	89	4-3-3 (6)			22.6					
5.0	624.0		brown and gray SILTY CLAY stiff	SS 3	78	3-3-5 (8)	1.5		25.3					
7.5	621.5		gray LEAN CLAY trace gravel stiff	SS 4	67	4-3-5 (8)	1.0		23.7					
10.0	620.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 630 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING 3.50 ft / Elev 626.50 ft

AT END OF DRILLING 3.50 ft / Elev 626.50 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

BORING NO. B-4

GEOCON PROFESSIONAL SERVICES

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	631.5		4" - 6" TOPSOIL											
			brown LEAN CLAY with sand medium stiff	SS 1	78	5-4-6 (10)	0.5	0.5	17.1					
2.5	628.5		brown and gray SANDY CLAY very soft to medium stiff	SS 2	56	3-4-3 (7)	0.75	0.8	20.9					
5.0			trace silt in SS3	SS 3	89	2-2-2 (4)	<0.25		26.3					
7.5	623.5		gray SILT, wet medium dense	SS 4	100	4-4-7 (11)			21.2					
10.0	622.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 632 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING ---

AT END OF DRILLING 8.50 ft / Elev 623.50 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

GEOCON PROFESSIONAL SERVICES

BORING NO. B-5

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	632.7		4" CONCRETE											
	632.5		2" GRAVEL FILL											
			brown and gray SANDY CLAY stiff	SS 1	78	8-6-5 (11)	1.0		14.6					
2.5	629.5		brown and gray LEAN CLAY trace sand stiff	SS 2	100	5-4-4 (8)	1.0		26.0					
5.0	627.0		brown and gray SILTY CLAY with sand meidum stiff	SS 3	89	3-4-8 (12)	0.5		22.3					
7.5	624.5		gray LEAN CLAY trace sand medium stiff	SS 4	100	3-3-3 (6)	0.5	0.6	22.5					
10.0	623.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 633 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

▽ **AT TIME OF DRILLING** 8.00 ft / Elev 625.00 ft

▼ **AT END OF DRILLING** 8.00 ft / Elev 625.00 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

BORING NO. B-6

GEOCON PROFESSIONAL SERVICES

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	633.5		4" - 6" TOPSOIL											
			dark brown, brown, and gray SANDY CLAY medium stiff	SS 1	78	4-3-4 (7)	0.5		18.2					
2.5	630.5		brown and gray LEAN CLAY stiff	SS 2	100	3-3-7 (10)	1.0	1.0	22.4					
5.0	628.0		▼ brown POORLY-GRADED SAND, wet medium dense	SS 3	89	5-7-8 (15)			26.1					
7.5	625.5		gray SILT, wet with clay medium dense	SS 4	56	3-4-6 (10)			20.7					
10.0	624.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 634 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

▽ **AT TIME OF DRILLING** 6.50 ft / Elev 627.50 ft

▼ **AT END OF DRILLING** 6.50 ft / Elev 627.50 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

22774 Citation Road, Unit A Frankfort, IL 60423 Phone 815-806-9986

GEOCON PROFESSIONAL SERVICES

BORING NO. B-7

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	632.0		4" - 6" TOPSOIL											
			brown and gray SANDY CLAY very stiff	SS 1	67	4-4-6 (10)	2.0	2.0	19.9					
2.5	629.0		brown and gray POORLY-GRADED SAND, very moist medium dense	SS 2	78	7-7-7 (14)			18.7					
5.0	626.5		brown and gray SILTY CLAY very stiff	SS 3	78	2-3-5 (8)	2.5	2.5	22.0					
7.5	624.0		gray POORLY-GRADED SAND, wet medium dense	SS 4	89	7-9-9 (18)			27.3					
10.0	622.5		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 632.5 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

▽ **AT TIME OF DRILLING** 8.50 ft / Elev 624.00 ft

▼ **AT END OF DRILLING** 8.50 ft / Elev 624.00 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

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GEOCON PROFESSIONAL SERVICES

BORING NO. B-8

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

DATE COMPLETED 6/4/26 **LOGGED BY** TH/BG **DRILLING METHOD** 3.25 in. HSA

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	633.5		4" - 6" TOPSOIL											
			brown POORLY-GRADED SAND trace clay loose	SS 1	56	3-4-5 (9)			12.6					
2.5	630.5		▼											
			brown and gray SILTY SAND, wet loose	SS 2	67	3-3-2 (5)			18.1					
5.0	628.0													
			brown and gray LEAN CLAY, wet medium stiff	SS 3	56	1-2-2 (4)	0.5		25.8					
7.5	625.5													
			brown and gray POORLY-GRADED SAND, wet medium dense	SS 4	78	4-6-12 (18)			19.9					
10.0	624.0		Bottom of borehole at 10.0 feet.											

COMPLETION DEPTH 10 ft **GROUND ELEVATION** 634 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

▽ **AT TIME OF DRILLING** 3.50 ft / Elev 630.50 ft

▼ **AT END OF DRILLING** 3.50 ft / Elev 630.50 ft

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

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ATTERBERG LIMITS RESULTS

GEOCON PROFESSIONAL SERVICES

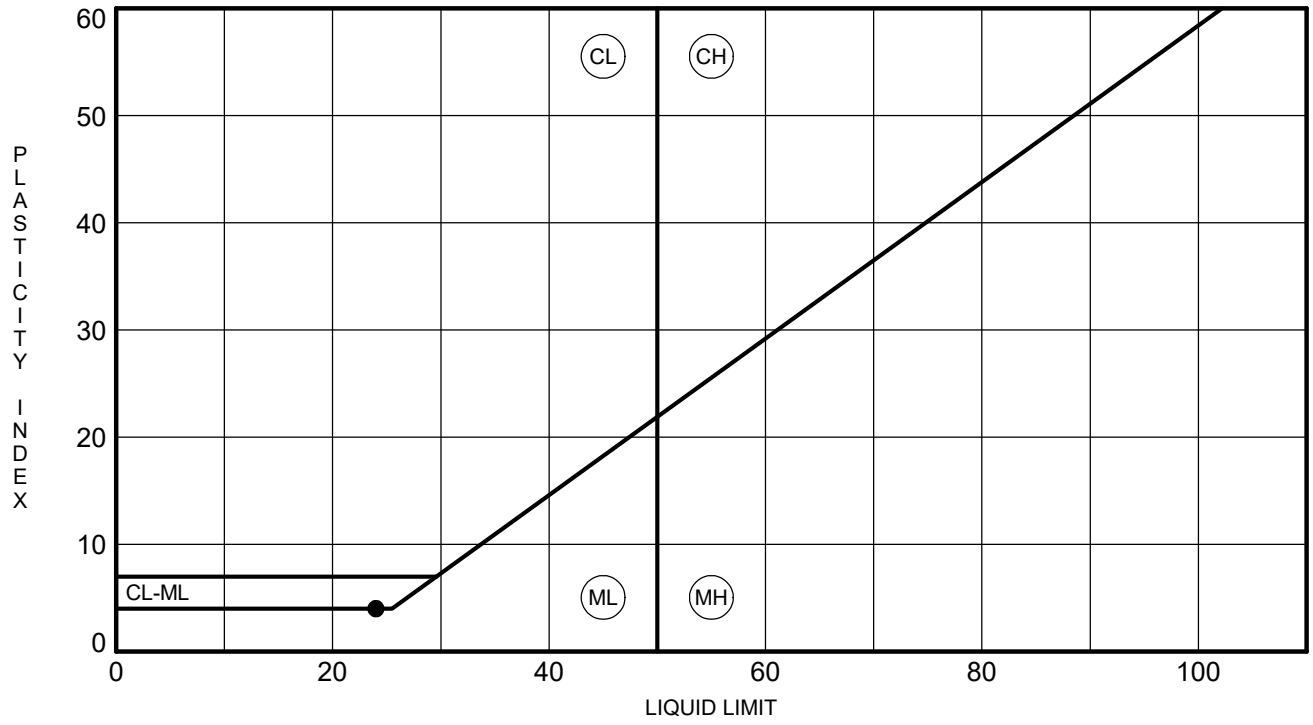
PRINT DATE 6/11/2026

CLIENT Construction Field Services, LLC

PROJECT NAME Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856

PROJECT LOCATION 2211 E 10th St, Hobart, IN 46342



	BOREHOLE	DEPTH	LL	PL	PI	Fines	Classification
●	**B-2**	**6.0**	**24**	**20**	**4**		

GEOCON PROFESSIONAL SERVICES

GENERAL NOTES

CLIENT Construction Field Services, LLC **PROJECT NAME** Hobart High School Baseball & Softball Fields

PROJECT NUMBER 26-G0856 **PROJECT LOCATION** 2211 E 10th St, Hobart, IN 46342

SAMPLE IDENTIFICATION

Visual soil classifications are made in general accordance with the United Soil Classification System (USCS) on the basis of textural and particle size categorization, and various soil behavior characteristics. Visual classifications should be substantiated by appropriate laboratory testing when a more exact soil identification is required to satisfy specific project applications criteria.

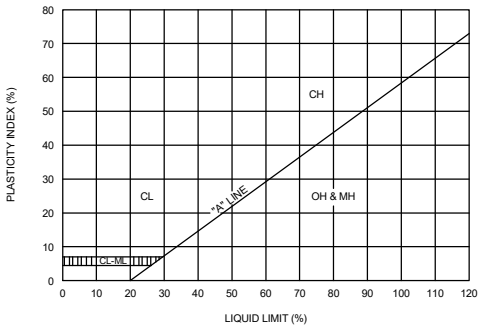
UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D-2487-98)

MATERIAL TYPES	CRITERIA FOR ASSIGNING SOIL GROUP NAMES			GROUP SYMBOL	SOIL GROUP NAMES & LEGEND
COARSE-GRAINED SOILS >50% RETAINED ON NO. 200 SIEVE	GRAVELS >50% OF COARSE FRACTION RETAINED ON NO 4. SIEVE	CLEAN GRAVELS <5% FINES	$C_u \geq 4$ AND $1 \leq C_c \leq 3$	GW	WELL-GRADED GRAVEL
			$C_u \geq 4$ AND/OR $1 \geq C_c \geq 3$	GP	POORLY-GRADED GRAVEL
		GRAVELS WITH FINES >12% FINES	FINES CLASSIFY AS ML OR CL	GM	SILTY GRAVEL
			FINES CLASSIFY AS CL OR CH	GC	CLAYEY GRAVEL
	SANDS >50% OF COARSE FRACTION PASSES ON NO 4. SIEVE	CLEAN SANDS <5% FINES	$C_u \geq 6$ AND $1 \leq C_c \leq 3$	SW	WELL-GRADED SAND
			$C_u \geq 6$ AND/OR $1 \geq Cc \geq 3$	SP	POORLY-GRADED SAND
		SANDS AND FINES >12% FINES	FINES CLASSIFY AS ML OR MH	SM	SILTY SAND
			FINES CLASSIFY AS CL OR CH	SC	CLAYEY SAND
FINE-GRAINED SOILS >50% PASSES NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT<50	INORGANIC	PI>7 AND PLOTS>"A" LINE	CL	LEAN CLAY
			PI>4 AND PLOTS<"A" LINE	ML	SILT
		ORGANIC	LL (oven dried)/LL (not dried)<0.75	OL	ORGANIC CLAY OR SILT
	SILTS AND CLAYS LIQUID LIMIT>50	INORGANIC	PI PLOTS >"A" LINE	CH	FAT CLAY
			PI PLOTS <"A" LINE	MH	ELASTIC SILT
		ORGANIC	LL (oven dried)/LL (not dried)<0.75	OH	ORGANIC CLAY OR SILT
HIGHLY ORGANIC SOILS		PRIMARILY ORGANIC MATTER, DARK IN COLOR, AND ORGANIC ODOR		PT	PEAT

PROJECT LITHOLOGIC SYMBOLS (USCS)

- CL: USCS Low Plasticity Clay
- CONCRETE: Concrete
- SM: USCS Silty Sand
- CL-ML: USCS Low Plasticity Silty Clay
- GRAVEL FILL: Gravel Fill
- SP: USCS Poorly-graded Sand
- CLS: USCS Low Plasticity Sandy Clay
- ML: USCS Silt
- TOPSOIL: Topsoil

PLASTICITY CHART



PROJECT SAMPLE TYPES

- Split Spoon (SS)

SOIL RELATIVE DENSITY AND CONSISTENCY CLASSIFICATION

NON-COHESIVE SOILS		COHESIVE SOILS		
RELATIVE DENSITY	N-VALUE*	CONSISTENCY	N-VALUE*	COMPRESSIVE STRENGTH (TSF)
VERY LOOSE	0 - 4	VERY SOFT	0 - 2	0 - 0.25
LOOSE	4 - 10	SOFT	2 - 5	0.25 - 0.50
MEDIUM DENSE	10 - 30	MEDIUM STIFF	5 - 10	0.50 - 1.0
DENSE	30 - 50	STIFF	10 - 14	1.0 - 2.0
VERY DENSE	OVER 50	VERY STIFF	14 - 32	2.0 - 4.0
		HARD	OVER 32	OVER 4.0

* N-VALUE: NUMBER OF BLOWS OF 140 LB HAMMER FALLING 30 INCHES TO DRIVE A 2 INCH O.D. (1-3/8 INCH I.D.) SPLIT-BARREL SAMPLER THE LAST 12 INCHES OF AN 18-INCH DRIVE (ASTM-1586 STANDARD PENETRATION TEST).

ABBREVIATIONS

SS	-	SPLIT-SPOON SAMPLE	LL	-	LIQUID LIMIT (%)
ST	-	SHELBY TUBE SAMPLE	PL	-	PLASTIC LIMT (%)
AU	-	AUGER SAMPLE	PI	-	PLASTIC INDEX (%)
MC	-	MOISTURE CONTENT (%)	NP	-	NON PLASTIC
-200	-	PERCENT PASSING NO. 200 SIEVE	DD	-	DRY DENSITY (PCF)
Qp	-	POCKET PENETROMETER (TSF)	DCP	-	DYNAMIC CONE PENETROMETER
Qu	-	UNCONFINED STRENGTH (TSF)	IBV	-	IMMEDIATE BEARING VALUE