

**ADDENDUM
NO. 1**

August 31, 2022

**EISENHOWER ELEMENTARY SCHOOL ADDITIONS, RENOVATIONS,
AND RELATED WORK
Crown Point, IN 46307**

TO: ALL BIDDERS OF RECORD

This Addendum forms a part of and modifies the Bidding Requirements, Contract Forms, Contract Conditions, the Specifications, and the Drawings dated August 18, 2022 by Gibraltar Design. Acknowledge receipt of the Addendum in the space provided on the Bid Form. Failure to do so may subject the Bidder to disqualification.

This Addendum consists of Pages ADD 1-1 and attached Addendum No. 1 from Gibraltar Design dated August 31, 2022 and consisting of 1 page and 5 drawings.

ADDENDUM ONE

Addendum One (AD.01) to the drawings and specifications prepared by Gibraltar Design for **Eisenhower Elementary School – Additions, Renovations and Related Work** for Crown Point Community School Corporation, Crown Point, Indiana.

All Contractors bidding on this project shall read all of the items covered below and shall comply with all of the requirements as set forth, including any necessary refinements or additions generated by this Addendum and required by the intent of the original contract documents. All Contractors shall acknowledge on their bid form that they have received this Addendum and include the appropriate content of same within their bid proposal.

DRAWINGS

1. Sheets S-001, S-201D, S-201E

A. Refer to three (3) revised, full size drawings, included in this Addendum, for revisions.

2. Sheet A-601

A. Refer to revised, full size drawing, included in this Addendum for the following revisions:

1. Aluminum Window Elevation 7: revised elevations.

3. Sheet FP001

A. Refer to revised, full size drawing, included in this Addendum for the following revisions:

1. Revised zoning plan.
2. Revised Ex. Fire Protection Service Diagram.

Page 1 and five (5) Full-Size Drawings, constitute the total makeup of **Addendum One**.





PROJECT

EISENHOWER ELEMENTARY SCHOOL ADDITIONS, RENOVATIONS, AND RELATED WORK

CROWN POINT COMMUNITY SCHOOL CORPORATION CROWN POINT, INDIANA

EXISTING CONSTRUCTION

- 1. The Contractor shall be responsible for complying with all safety precautions and regulations during the work.
2. The Structural Engineer of Record will not advise on, nor issue direction on safety precautions and programs.
3. The Structural Drawings herein represent the finished structure. The Contractor shall provide all necessary equipment and bracing required to erect and hold the structure in proper alignment until all Structural Work and connections have been completed.
4. Drawings indicate general and typical details of construction. Where conditions are not specifically shown, shop drawings, temporary supports, etc. are the sole responsibility of the Structural Engineer of Record.
5. All structural systems which are to be composed of components to be field erected shall be approved by the Supplier during manufacturing, delivery, handling, storage, and erection in accordance with the Supplier's instructions.
6. Loading applied to the structure during the process of construction shall not exceed the safe load-carrying capacity of the structure in any direction. The live loading used in the design of this structure are indicated in the Design Criteria Notes. Do not apply any construction loads until structural framing is properly connected together and until all temporary bracing is in place.
7. All ASTM and other referenced standards and codes are for the latest editions of these publications, unless otherwise noted.
8. Shop drawings and other items shall be submitted to the Structural Engineer of Record (SER) for review prior to fabrication. All Shop Drawings shall be reviewed by the Contractor before submission. The SER's review is for the best performance with the design concept and general compliance with the relevant Contract Documents. The SER's review does not relieve the Structural Engineer of Record of his responsibility to review, check, and coordinate the Shop Drawings prior to submission. The Contractor remains solely responsible for errors and omissions associated with the preparation of Shop Drawings as they pertain to member details, connections, and erection.
9. Submit Shop Drawings in the form of blue/blackbacked prints (min. 2 sets/ max. 5 sets) and one original blue/black or paper copy. In no case shall reproductions of the Contract Documents be substituted for the original. In a minimum, submit the following items for review:
A. Concrete Mix Designs.
B. Reinforcing Steel Shop Drawings.
C. Structural Steel Shop Drawings.
D. Steel Joist Shop Drawings.
E. Welding Specifications.
F. Cold-Formed Steel Framing Systems.
10. Resubmitted Shop Drawings. Resubmitted shop drawings are reviewed only in response to comments made in the previous submittal.
11. When calculations are included in the submittals for components of work designed and certified by a Specialty Structural Engineer (SSE), the review by the Structural Engineer of Record (SER) shall be for compliance with the relevant Contract Documents. The SER's review does not relieve the SSE from responsibility for the design of the system(s) and the coordination with the elements of the structure under the certification of the SER, or other SSE. The SER's review does not constitute a warranty of the accuracy or completeness of the SSE's design.
12. Contractors shall verify the size prior to bid to ascertain conditions which may adversely affect the work.
13. No structural member may be cut, notched, or otherwise reduced in strength without written direction from the Structural Engineer of Record.
14. When modifications are proposed to structural elements under the design and certification of a Specialty Structural Engineer (SSE), written authorization by the SSE must be obtained and submitted to the Structural Engineer of Record prior to performing the proposed modification.

GENERAL NOTES

DESIGN CRITERIA

- 1. DESIGN STANDARDS: The intended design standards and/or criteria are as follows:
General: The 2014 Indiana Building Code (2012 International Code with Indiana Amendments)
ACI 318
ASCE Manual, Allowable Stress Design (ASD)
Concrete: ACI 308
Steel: AISC Manual, Allowable Stress Design (ASD)
Steel Joists/Girders: Steel Joist Institute
Steel Deck: Steel Deck Institute
Cold-Formed Metal: AISI-S100
Wood Framing: NDS
Wood Trusses: TPI
Glulam Construction: ICC-ES ESR-1103
All referenced standards and codes, as well as ASTM numbers, are for the editions of these publications referenced in the Building Code listed above, unless otherwise noted.
2. DESIGN LOADS: Gravity Load Levels: Gravity Load Levels are as specified for the materials of construction incorporated into the building, including but not limited to walls, floors, ceilings, stairways, floor partitions, finishes, cladding and other similar architectural and structural items, as well as mechanical, electrical and plumbing equipment and fixtures, and material handling and live service equipment, including the weight of cranes.
3. LIVE LOADS: Gravity live loads used in the design of the structure meet, or exceed the following table (IBC 2012, 1607.1):

Table with columns: OCCUPANCY OR USE, UNIFORM (PSF), CONCENTRATED (LB/FOOT #1)

- A. Schools
1. Classrooms 40
2. First Floor Corridors 100
3. Corridors Above 1st Floor 80
Note #1: Unless otherwise noted, the indicated concentrated load has been assumed to be uniformly distributed over an area of 9' x 30' "x"
Note #2: The concentrated wheel load has been applied on an area of 20 sq. in.
4. PARTITION ALLOWANCE: A uniform partition allowance of 15 PSF has been used to account for the load of all floors where partition locations are subject to change, unless the specified live load exceeds 80 PSF.
5. COLLATERAL LOAD: Unless otherwise noted, a minimum uniform collateral load of 10 PSF has been used to account for ductwork, ceiling, sprinklers, lighting, etc. The collateral load is in addition to the weight of mechanical equipment, piping and greater than 4" diameter and suspended fixtures or equipment that have been specifically accounted for in the design.
6. COLLATERAL LOAD ABOVE CORRIDORS & MECHANICAL ROOMS: A minimum uniform collateral load of 10 PSF has been used to account for large ductwork, sprinkler mains, concentrations of piping and electrical distribution above corridors and mechanical rooms. The collateral load is in addition to the weight of mechanical units and large piping and greater than 4" diameter and suspended fixtures or equipment that have been specifically accounted for in the design.
7. CONCENTRATED LOADS: All single panel points of the lower chord of exposed roof trusses or any joint along the primary structural member supporting roof loads shall be capable of carrying safely a suspended concentrated load of not less than 200 LBS in addition to dead load.
8. HANDRAIL ASSEMBLIES AND GUARDS: 50 PLF applied in any direction (non-concurrent with 50 PLF load)
9. ROOF LIVE/SNOW LOADS: Gravity Live Loads used in the design of the roof structure meet or exceed the following table:
A. Snow Load:
Roof Snow Load, Pg 30 PSF
Flat Roof Snow Load, Pf 23 PSF
Slope Minimum Snow Load, Pm 10 PSF
Exposure Factor, Ce 1.0
Risk Category (IBC Table 1604.5) 1.1
Wind Importance Factor, Is 1.0
Thermal Factor, Ct 1.0
B. Minimum Roof Live Load: 20 PSF
C. Overhang Eaves & Projections: 44 PSF
1. Sloped roof snow loads calculated in accordance with Section 7.4, ASCE 7.
2. Unbalanced roof snow loads calculated in accordance with Section 7.6, ASCE 7. Specialty Structural Engineers must consider unbalanced snow loads in the design of pre-engineered structures, frames, skylights, curtain walls, cold-formed metal framing, canopies, etc.
3. Drift loads calculated in accordance with Section 7.7, ASCE 7.
4. Loads used for roof girders or assembly purposes have been designed for a minimum live load of 10 PSF.

CAST IN PLACE CONCRETE

- 1. Details of fabrication of reinforcement, handling and placing of the concrete, construction of forms and placement of reinforcement not otherwise covered by the Plans and Specifications, shall comply with the ACI Code requirements of the latest revised date.
2. Cold weather concreting shall be in accordance with ACI 308. Cold weather is defined as a period when for more than 3 successive days the average daily air temperature drops below 40F and stays below 50F. The Contractor shall maintain a copy of this information on site.
3. Hot weather concreting shall be in accordance with ACI 308. Hot weather is defined as any combination of the following conditions that tends to impair the quality of the freshly mixed or hardened concrete: high ambient temperature, high concrete temperature, low relative humidity, wind speed, or solar radiation. The Contractor shall maintain a copy of this information on site.
4. A certified Testing Agency shall be retained to perform industry standard testing including measurement of slump, air temperature, concrete finishing and testing, etc. to ensure conformance with the Contract Documents. Submit reports to Architect/Engineer.
5. Finishing of Slabs: After screeding, bid coating and finishing operations have been completed, apply finish film as indicated below, and as described in the Division 3 Cast in Place Concrete Specification of the Project Manual.
A. Floor Slabs: Hard Trowel Finish
B. Ramps, Stairs, & Sidewalks: Boom Finish
C. Surfaces to Receive Topping Slab: Float Finish
D. Surfaces to Receive thick-set mortar: Float Finish
E. Driving Surfaces: Rough Smooth Finish
Sample Finishes: See Specifications for sample and mockup requirements, if any.
6. Floor Tolerances: See the Specifications for specified FF and FI tolerances. FF and FI testing shall be performed by the Testing Agency in accordance with ASTM E-1165. Results, including acceptance or rejection of the work to be provided to the Contractor and the Architect/Engineer within 48 hours after data collection. Remedies for out-of-tolerance work shall be in accordance with the Specifications.
7. When spreading of reinforced steel is prohibited, measurement of the gaps between a 1/8" flat straight edge may be used in lieu of FF and FI testing. Approval must be obtained in writing prior to the beginning of concrete operations.
8. Finishing of Formed Surfaces: Finish forms surfaces as indicated below, and as described in the Division 3 Cast in Place Concrete Specification of the Project Manual.
A. Sides of Footings & Pile Caps: Rough Form Finish
B. Sides of Grade Beams: Rough Form Finish
C. Surfaces not exposed to public view: Rough Form Finish
D. Surfaces exposed to public view: Smooth Form Finish
9. The Contractor shall consult with the Structural Engineer of Record before starting concrete work to establish a satisfactory placing schedule and to determine the location of construction joints so as to minimize the effects of shrinkage in the form systems.
10. Sawn or coated formwork panels shall be provided in all slabs on grade. For a framed structure, joints shall be located on all column lines. If the column spacing exceeds 20'-2", provide intermediate joints. Vertical joints, and minor slabs without column shall have joints spaced a maximum of 15'-0" apart. Layoff joints shall be made at maximum aspect ratio (rate of long side to short side) not to exceed 1:3.
11. Where vinyl composition tile, vinyl sheeting, gobs, self-etch epoxy render, or other similar material is the specified finish floor material, the Contractor shall coordinate the locations of contraction and construction joints with the Finish Flooring Contractor. Submit a dimensional plan showing joint locations and proposed sequence of floor pours.
12. Unless specifically noted on the Plans, composite and non-composite supported slabs on metal deck, and supported cast-in-place concrete slabs do not require sawn control joints.
13. Joints in slabs to receive a finished floor may remain unsealed, unless required by the finish flooring contractor. All exposed slabs shall be filled with sealant specified in Division 7, or as follows: All slabs in industrial, manufacturing, or warehouse applications subject to wheeled traffic shall be filled with specified epoxy resin sealant. All other joints shall be filled with specified elastomeric sealant. Deter fluffing of joints as long as possible, preferably a minimum of 4 to 6 weeks after the slabs have been cured. Prior to fluffing, remove all debris from the slab joints. The fluff in accordance with the manufacturer's recommendations.

CONCRETE REINFORCING

- 1. Reinforcement, other than cold drawn wire for spirals and welded wire fabric, shall have deformed surfaces in accordance with ASTM A305.
2. Reinforcing steel shall conform to ASTM A615, Grade 60, unless noted.
3. Welded wire fabric shall conform to ASTM A1064, unless noted.
4. Where hooks are indicated, provide standard hooks per ACI and CRSI for all bars unless other dimensions are shown on the drawings or details.
5. Reinforcement in footings, walls and beams shall be continuous. Lap bars a minimum of 40 diameters, unless noted otherwise.
6. Reinforcement shall be supported and secured against displacement in accordance with the CRSI Manual of Standard Practice.
7. Details of reinforcing steel fabrication and placement shall conform to ACI 315 Details and Detailing of Concrete Reinforcement. The Contractor shall maintain a copy of this information on site.
8. Spread reinforcing steel around small openings and sleeves in slabs and walls, where possible, and where bar spacing will not exceed 15 diameters, unless otherwise noted. Discourage bars at all large openings where necessary, and provide an area of reinforcement, equal to the interrupted reinforcement, in full length bars, distributing one-half each side of the opening. Where shrinkage and temperature reinforcement is interrupted, weld (2) #5's opening dimension < 4" on each side of the opening. Provide #5's < 4" long diagonal bars in both faces, at each corner of openings larger than 12" in any direction.
9. Provide standards for the support of reinforcement for footings, pile caps, and mats.
10. Provide individual high chairs with support bars, as required for the support of top reinforcement for supported slabs. Do NOT provide standards.
11. Provide snap-on plastic spacers within a maximum required concrete cover for vertical wall reinforcement.
12. Where walls are cast in column footings, provide dowels for the wall. Dowels shall be the same size and spacing as the vertical wall reinforcement, with lap splices as shown on the application sections. Install dowels in the footing forms before concrete is placed. Do NOT stick dowels into footings after concrete is placed.
13. When spreading of reinforced steel is prohibited, measurement of the gaps between a 1/8" flat straight edge may be used in lieu of FF and FI testing. Approval must be obtained in writing prior to the beginning of concrete operations.
14. Minimum concrete cover for reinforcing steel shall be as follows, unless noted otherwise on plan, section or note:
MINIMUM COVER FOR REINFORCEMENT

Table with columns: WALLS, MINIMUM COVER, FOR DRY CONDITIONS, FORMED CONCRETE SURFACES EXPOSED TO EARTH, WATER, SEWAGE, WEATHER, OR IN CONTACT WITH GROUND, TOP OF FOOTINGS, OVER TOP OF PILES

FOUNDATIONS

- 1. Proofslab on grade areas with a medium-weight roller or other suitable equipment to check for voids, including but not limited to:
A. Column Sockets.
B. Moment Connections.
C. Bracing Connections including Collectors and Drag Struts.
D. Welded Shear Connections.
2. All beam-to-beam connections shall be double angle, unless shown or noted otherwise.
3. All beam-to-column connections shall be at the column centerline, unless noted otherwise. Shear tab connections in tubes are permitted unless otherwise noted.
4. Bolted connections in moment frames, tension connections, hangers and stud columns, crane connections, and those designated PT (pretensioned) on the Drawings shall be pretensioned joints utilizing tension-control (TC) bolts of direct tension indicators. Bolts for PT bolts shall be 1/16" larger than the bolt diameter. All pretensioned joints shall be inspected by the Testing Agency.
5. Connect bracing members for two components of stress unless otherwise approved by the Structural Engineer of Record. Provide a minimum 2-bolt or welded field connection.
6. Column centerlines of all vertical bracing members on column centerlines in vertical plane and on local and all centerlines in horizontal plane, unless otherwise shown on the Structural Drawings.
7. All welding shall be in accordance with AWS D1.1 using E70XX electrodes, unless shown or noted otherwise. Welding, both shop and field, shall be performed by welders certified for the weld types and positions involved according to the current edition of AWS D1.1. Perform all AWS tests with care to provide a clean, uniform appearance.
8. Backup bars required for welded connections shall be continuous.
9. Holes in steel shall be drilled or punched. All drilled holes shall be provided with smooth edges. Burring of holes in structural steel shall be not allowed without approval of the Structural Engineer of Record.
10. The minimum thickness of all connection material shall be 5/16" unless noted.
11. Continuous bent plates and angle closures, rod, welded diaphragm chords, and/or prandtl members of the roof and roof, as well as around openings shall be welded with a minimum 1/4" fillet welds x 2" long at 12" o.c., top & bottom, unless noted otherwise. But weld joints in continuous diaphragm chord for continuity. For continuous perimeter members and/or connections to and connected to the top chords of joists, provide a minimum 3" d 1/4" weld at each joint. Continuous angle and bent plate closures may be shop-applied to the supporting structural members only when requested and approved by Structural Engineer of Record.
12. Where steel beams are called to have wood nailers supporting wood floor or roof framing, provide 1/2" diameter cambrag bolts at 24" on center and staggered each side of the beam web, unless noted otherwise. Cambrag bolts may be over-height to compress the rounded head to the nailer to facilitate installation of continuous nailing joints, rafters, trusses, etc.
13. A qualified independent Testing Agency shall be retained to perform inspection and testing of structural steel weldments as follows:
WELD INSPECTION SCHEDULE

Table with columns: WELD TYPE, VT, MT, UT, PT, CRT, COMMENTS

POST-INSTALLED DOWELS & ANCHOR BOLTS/RODS

- 1. All reinforcing steel and threaded rod anchors to be installed in a 2-part chemical anchoring system shall be treated as follows:
A. Drill holes larger than bar or rod to be embedded. Coordinate hole diameter with Manufacturer's recommendations.
B. Holes must be cleaned and prepared in accordance with Manufacturer's recommendations.
C. When reinforcing steel is encountered during drilling for installation of anchors, stop drilling, use a sensor to locate the reinforcing in the surrounding area and install anchor(s) as close as possible to the original location. Contact the Structural Engineer of Record (SER) for direction when the revised location is more than 2" from the original location, or when the original location of the anchorage is significantly altered. When in doubt, contact the SER for direction.
D. Drill the hole a minimum of 1/2 bar diameters or as shown on the plans.
E. Use a 2-part adhesive anchoring system, Hilti HY-200, or approved equal.
F. For anchorage into hollow substrate, use Hilti HY-270, or approved equal.
G. Reinforcing steel dowels shall be ASTM A615, Grade 60, unless noted.
H. Anchor rods shall be Hilti HAS-V38, unless noted. Provide finish as noted on the Drawings. If not noted, provide hot-dip galvanized finish for interior applications. Provide stainless steel finish for all exterior applications, unless noted.
2. When column anchor bolts have been omitted, or damaged by construction operations, the Contractor must obtain the written approval of the Structural Engineer of Record prior to repair or replacement.
A. As a precaution, the affected column must be grouted and braced after repair for the balance of the erection period.
B. As an alternate to grouting and bracing, the Contractor may, at his option, employ a testing agency to perform a tensile pull test to confirm the strength for the repaired or replaced anchor bolt. The tensile pull test must exceed 1.33 x the design load of the original anchor without causing distress of the anchor bolt or the surrounding concrete. Reference the following table for the minimum proof loads:
3/4" diameter: 12.8 kips
7/8" diameter: 17.4 kips
1" diameter: 22.7 kips
1 1/8" diameter: 28.8 kips
1 1/4" diameter: 35.8 kips
Note: Values listed above are for ASTM F-1554, Grade 36 Material. When higher grade or strength materials are specified, refer to the AISI Steel Design Guide 1, Table 3.1 for minimum allowable loads to be multiplied by 1.33.
C. When affected anchor bolts are part of a fixed moment resisting column base, such as those in moment-resisting space frames, canopies, or knee-base installations, the repaired anchor bolts must be proof loaded, or the affected column footing and/or pier replaced in its entirety.
D. When affected anchor bolts are part of a braced frame the affected column footing and/or pier must be replaced in its entirety.
E. In addition to inspection requirements for fillet welds in Table above, 100% of field welding of diagonal bracing members to gusset plates shall be visually inspected (VT).

STEEL DECK NOTES

- 1. All steel deck material, fabrication and installation shall conform to the Steel Deck Institute "SDI SHORT FORM SPECIFICATIONS" and "SDI CODE OF STANDARD PRACTICE," current edition, unless noted.
2. Provide members for deck support at all deck span lengths. Provide L3x3x3/16 deck support at all columns where required.
3. All decks shall be provided in a minimum of 2-span lengths where possible.
4. All welding of steel deck shall be in accordance with AWS Specification D1.3. Provide welding washers for all floor decks less than 20 gauge in thickness.
5. Mechanical fasteners may be used in lieu of welding, providing fasteners meet, or exceed the strength of specified welds. Submit fastener design data to the Structural Engineer of Record for review.
6. Substitution of floor secondary reinforcement for welded wire fabric or approved slabs is prohibited.
7. Do not suspend any items, such as ductwork, mechanical and electrical fixtures, ceilings, etc. from steel deck.
8. Steel deck sidewalks shall be attached at ends of cantilevers and at a maximum spacing 12' o.c. from cantilevered roof deck ends. The roof deck members shall be completely fastened to the supports and the sidewalks before any load is applied to the cantilever.
9. Submit shop drawings for review of general conformance to design concept in accordance with Specifications in the Project Manual. Erection drawings shall show type of deck, shop finish, accessories, method of attachment, edge details, deck openings and reinforcement, and sequence of installation. Installation holes shall be sealed with a closure plate 2 gauges thicker than deck and mechanically fastened to deck. Steel deck holes visible from below will be repaired. Deck units are a bent, warped, or damaged in any way which would impair the strength and appearance of the deck shall be removed from the site.
10. Where gage metal protrusions are indicated, supply protrusions designed to meet, or exceed the gage metal in the SDI Flange Selection Table (min. 1/8" gage) in accordance with steel depth, concrete weight, and cantilever distance, unless noted otherwise.
11. The Erector shall install parallel roof beams and joists with differential mill and induced cambers to level deck bearing.

STEEL CONNECTION NOTES

- 1. Typical beam-to-beam and beam-to-column connections shall be bearing type welded joints, unless noted otherwise.
2. Shop connections unless otherwise shown, may be either bolted or welded. All connections shall be bolted unless otherwise shown on the Structural Drawings.
3. Connections shall be designed by the Steel Fabricator to support the reactions shown on the framing plan(s). Simple span connections shall be designed on the Structural Drawings shall be designed by the Steel Fabricator in accordance with Table 3-6 of the AISC "Manual of Steel Construction, 14th Edition". For composite beams where reactions are not indicated, design connections for 75% of the Maximum Total Uniform Load (ASD) values for the beam size and span given in Table 3-6. For non-composite beams, design connections for 50% of the tabulated ASD value.
4. Submit calculations for connections not detailed on the Structural Drawings and not covered by the AISI Tables, including but not limited to:
A. Column Sockets.
B. Moment Connections.
C. Bracing Connections including Collectors and Drag Struts.
D. Welded Shear Connections.
5. All beam-to-beam connections shall be double angle, unless shown or noted otherwise.
6. All beam-to-column connections shall be at the column centerline, unless noted otherwise. Shear tab connections in tubes are permitted unless otherwise noted.
7. Typical beam-to-beam, and beam-to-column field-bolted connections may be tightened to the snug-tight condition, unless otherwise shown or noted.
8. Bolted connections in moment frames, tension connections, hangers and stud columns, crane connections, and those designated PT (pretensioned) on the Drawings shall be pretensioned joints utilizing tension-control (TC) bolts of direct tension indicators. Bolts for PT bolts shall be 1/16" larger than the bolt diameter. All pretensioned joints shall be inspected by the Testing Agency.
9. Connect bracing members for two components of stress unless otherwise approved by the Structural Engineer of Record. Provide a minimum 2-bolt or welded field connection.
10. Column centerlines of all vertical bracing members on column centerlines in vertical plane and on local and all centerlines in horizontal plane, unless otherwise shown on the Structural Drawings.
11. All welding shall be in accordance with AWS D1.1 using E70XX electrodes, unless shown or noted otherwise. Welding, both shop and field, shall be performed by welders certified for the weld types and positions involved according to the current edition of AWS D1.1. Perform all AWS tests with care to provide a clean, uniform appearance.
12. Backup bars required for welded connections shall be continuous.
13. Holes in steel shall be drilled or punched. All drilled holes shall be provided with smooth edges. Burring of holes in structural steel shall be not allowed without approval of the Structural Engineer of Record.
14. The minimum thickness of all connection material shall be 5/16" unless noted.
15. Continuous bent plates and angle closures, rod, welded diaphragm chords, and/or prandtl members of the roof and roof, as well as around openings shall be welded with a minimum 1/4" fillet welds x 2" long at 12" o.c., top & bottom, unless noted otherwise. But weld joints in continuous diaphragm chord for continuity. For continuous perimeter members and/or connections to and connected to the top chords of joists, provide a minimum 3" d 1/4" weld at each joint. Continuous angle and bent plate closures may be shop-applied to the supporting structural members only when requested and approved by Structural Engineer of Record.
16. Where steel beams are called to have wood nailers supporting wood floor or roof framing, provide 1/2" diameter cambrag bolts at 24" on center and staggered each side of the beam web, unless noted otherwise. Cambrag bolts may be over-height to compress the rounded head to the nailer to facilitate installation of continuous nailing joints, rafters, trusses, etc.
17. A qualified independent Testing Agency shall be retained to perform inspection and testing of structural steel weldments as follows:
WELD INSPECTION SCHEDULE

STEEL JOIST NOTES

- 1. All steel joists shall be designed, fabricated, and erected in accordance with SJI Standard Specifications.
2. Joist bridging (if shown) is schematically indicated. Provide all bridging necessary to conform to SJI Specifications.
3. The ends of all bridging lines terminating at walls or beams shall be anchored to the wall or beam.
4. Joist bridging and railing shall be completely installed prior to placing any construction loads on the joists. Construction loading shall not exceed the joist design load.
5. All roof joists shall be capable of resisting the net loads noted on the Structural Drawings (min. 15 psf net). Provide an additional row of continuous horizontal chord bridging at the first panel point location at each end of all roof joists.
6. Special joists (SJ) shall be designed for the load designations specified on the Structural Drawings. Designs shall properly account for the distribution of concentrated loads, live loads, and for the effect of openings. Designs are not the meeting requirements of SJI.
7. Joists shall meet the following deflection criteria per SJI. Maximum live load deflection shall not exceed:
A) Roofs without suspended ceilings: L/240
B) Roof with suspended ceilings: L/300
8. The Joist Manufacturer shall submit calculations for all special joists to Structural Engineer of Record for review purposes prior to fabrication. These calculations shall bear the seal and signature of a Professional Engineer registered in the State of Indiana.
9. Joists on column centerlines shall have extended bottom chord connections for erection stability, unless otherwise noted. Do not connect bottom chord extensions, unless otherwise noted or shown. Joists on, or near column centerlines shall have field-bolted connections for erection stability, unless otherwise noted.
10. The Joist Manufacturer shall coordinate with the Structural Steel Fabricator for the design of all connections to support columns, beams, bearing seats, etc. prior to submittal of shop drawings.
11. Where a joint is part of a moment-resisting frame, slay the connection of the bottom chord to the column until all dead loads have been placed. All field-bolted and field-welded connections in moment-resisting frames shall be inspected per AWS and ASCE requirements.
12. The Joist Manufacturer shall furnish evidence that the joint meets or exceeds minimum moment of inertia (Iy) required to meet the specified loads and deflection criteria.
13. All steel joists shall be furnished with standard SJI cambers, unless noted otherwise.
14. All items suspended from joists such as catwalks, basketball goals, overhead partitions, etc. should be installed after all dead loads of roofing, flooring, ceilings, etc. are installed.
15. All joists shall be shop primed in accordance with SJI requirements, unless noted otherwise. Color to match structural steel color, unless approved in writing.
16. Provide spaced bearing ends at joint space exceeds 14".
17. Do not field cut or alter joists without the written approval of the Joist Manufacturer.

INTEL SCHEDULE

- 1. Where lintels are not specifically shown or noted on the Structural or Architectural Drawings, provide the following lintels at all openings and recesses in both interior and exterior non-load-bearing walls.
A) Brick:
Masonry Opening: Angle Size L5x5x3/16
Up to 5" x 4" up to 7'-0" L5x3x8
Over 5'-0" x 4" up to 7'-0" L5x3x8
Over 7'-0" L5x3x8 w/ Plate (see detail below)
WELDS TO BE 3" LONG, 0" o.c.
LONG LOOSE LINTEL DETAIL
All angles are LLL (long leg vertical), unless noted otherwise. Provide 1" of bearing per foot of span except with minimum 8". All lintels in exterior walls are to be hot-dip galvanized.
B) Block:
For openings up to 6'-0" long exposed in the finished room, use lintel blocks filled with grout. End all exposed joints and reinforce as follows:
1) For 6" thick block: 1 - #6 bar
2) For 8" thick block: 2 - #6 bars
3) For 10" thick block: 2 - #8 bars
4) For 12" thick block: 2 - #8 bars
C) Block: For openings over 6'-0" x 4" up to 12'-0" long exposed in the finished room, use lintel block filled with grout. Grout all exposed joints and reinforce per the "Long Masonry LinTEL Detail" on the Typical Masonry Detail Drawing.
D) Block (dead load openings over 4'-0"): See framing plans for steel beam lintels. Where not shown on plan, the criteria in the following table shall be used. Contact Structural Engineer of Record for lintels not shown on plan which do not meet this criteria. See architectural drawings for opening quantities, sizes, locations, heights of wall above, etc.
Block Y LINTEL WIDTH OF OPENING MAX ALLOW HEIGHT OF CMU ABOVE LINTEL

Table with columns: Block Y, LINTEL, WIDTH OF OPENING, MAX ALLOW HEIGHT OF CMU ABOVE LINTEL

- 1) For 6" thick block: 1 - #6 bar
2) For 8" thick block: 2 - #6 bars
3) For 10" thick block: 2 - #8 bars
4) For 12" thick block: 2 - #8 bars
C) Block: For openings over 6'-0" x 4" up to 12'-0" long exposed in the finished room, use lintel block filled with grout. Grout all exposed joints and reinforce per the "Long Masonry LinTEL Detail" on the Typical Masonry Detail Drawing.
D) Block (dead load openings over 4'-0"): See framing plans for steel beam lintels. Where not shown on plan, the criteria in the following table shall be used. Contact Structural Engineer of Record for lintels not shown on plan which do not meet this criteria. See architectural drawings for opening quantities, sizes, locations, heights of wall above, etc.
Block Y LINTEL WIDTH OF OPENING MAX ALLOW HEIGHT OF CMU ABOVE LINTEL

COORDINATION WITH OTHER TRADES

- 1. The Contractor shall coordinate and check all dimensions relating to electrical finishes, mechanical equipment and openings, elevators shafts and overruns, etc. and notify the Architect/Engineer of any discrepancies before proceeding with any work in the area under review.
2. The Structural Drawings shall be used in conjunction with the Drawings of all other disciplines and the Specifications. The Contractor shall verify the requirements of other trades as to doors, trusses, hangers, inserts, anchors, holes, and other items to be placed or set in the Structural Work.
3. There shall be no vertical or horizontal sleeves set, or holes cut or drilled in any beam or column unless it is shown on the Structural Drawings or approved in writing by the Structural Engineer of Record.
4. Mechanical and electrical openings through supported slabs and walls, 8" diameter or larger, not shown on the Structural Drawings must be approved by the Structural Engineer of Record (SER). Openings less than 8" diameter shall have at least 1'-0" clear between openings, unless approved in writing by the SER.
5. Verify locations and dimensions of mechanical and electrical openings through supported slabs and walls shown on the Structural Drawings with the Mechanical and Electrical Contractors.
6. Do not install conduit in supported slabs, slabs on grade, or concrete walls unless explicitly shown or noted on the Structural Drawings.
7. Do not suspend any items, such as ductwork, mechanical or electrical fixtures, ceilings, etc. from steel roof deck or wood roof sheathing.
8. The Mechanical Contractor shall verify that mechanical units supported by the steel framing are capable of spanning the distance between the supporting members indicated on the Structural Drawings. The Mechanical Contractor shall supply additional supporting as required.
9. If drawings and specifications are in conflict, the most stringent restrictions and requirements shall govern.

COLD-FORMED (LIGHT GAUGE) METAL FRAMING NOTES

- 1. All cold-formed steel framing members, their design, fabrication, and erection shall conform to the "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" of the latest edition of the AISI.
2. All framing members shall be formed from steel conforming to ASTM A653, with a minimum yield strength as follows:
A) All framing members shall be formed from steel conforming to ASTM A653, with a minimum yield strength as follows:
A) 12, 14 & 16 gage members: Fy=50ksi
B) 18, 20 gage members: Fy=33ksi
3. All framing members shall be galvanized with a G60 coating meeting the requirements of ASTM A653, unless otherwise indicated.
4. Members shall be the Manufacturer's standard "C"-Shaped studs/ joists of the size, flange width, and gage indicated. All members shall have a minimum flange to flange of 12" and satisfy the minimum properties in accordance with the Steel Framing Manufacturers Association (SFMA) standards.
5. The gage of all tracks shall match the gage of the associated stud or joist, unless otherwise noted.
6. All welding shall be in accordance with AWS Specification D1.3. No welding of members less than 14 gage in thickness is permitted without the approval of the SER. All welding shall be performed by certified welders. All welds shall be touched up with zinc rich paint in accordance with ASTM A780.
7. Provide bridging for all load-bearing wall studs at a maximum spacing of 48" on center.
8. One row of bridging within 18" of the top tracking when a single deep deflection track is utilized.
9. All axial-loaded studs shall have full bearing against the track web, prior to stud and track alignment.
10. Provide the Manufacturer's standard track, clip angles, bracing, reinforcement, fasteners, and accessories as recommended by the Manufacturer for the application indicated and as needed to provide a complete framing system. Unless otherwise indicated, install the metal framing system in accordance with the Manufacturer's shop drawings, written instructions and recommendations.
11. Install supplementary framing, blocking, and bracing in metal framing system wherever walls or partitions are indicated to support loads, equipment, services, canopy, heavy trim and furnishings, and similar work requiring attachment to the wall or partition. Where type of supplementary support is not otherwise indicated, comply with the stud manufacturer's recommendations and industry standards in each case, considering weight or loading resulting from the item supported.
12. All field-cutting of studs must be done by sawing or shearing. Torch-cutting of cold-formed members is not allowed.
13. No notching or coping of studs is allowed, unless explicitly shown on the design or shop drawings. All field-cut holes must be reinforced.
14. The Framing Contractor is to ensure proper alignment when assembling lateral bracing/bridging and field-cutting studs to length. Lateral bracing/bridging must be installed at the time the wall is erected.
15. Temporary bracing shall be provided and remain in place until work is completely stabilized.
16. Use a minimum of three studs at the corners of all exterior walls.
17. All headers and built-up beams must be composed of UNLINED material only. Install insulation in built-up exterior framing members, such as headers, sills, bowed joists, and field members at openings, that are inaccessible on completion of framing work.
18. Shop drawings: Show layout, spacing, size, thickness, type of cold-formed metal framing, and fastening and anchorage details, including mechanical fasteners. Show reinforcing channels, opening framing, supplemental framing, strapping, bracing, joists, accessories, connection details, and attachment to supporting work.
19. For cold-formed metal framing indicated to comply with design loads, include structural analysis data signed and sealed by the qualified professional engineer (SSE) responsible for their preparation.
20. Structural Performance: Provide cold-formed metal framing capable of withstanding design loads with limits and under conditions indicated.
A) Design Loads: Reference the Design Criteria Notes.
B) Deflection Limits: Design framing systems to withstand design loads without deflections greater than the following:
1. Wall Framing: Horizontal deflection of 1/240 of the wall height for walls with flexible finishes, e.g. metal siding, wood siding, EIFS, etc.
2. Wall Framing: Horizontal deflection of 1/360 of the wall height for walls with cementitious finishes, e.g. cement plaster.
3. Wall Framing: Horizontal deflection of 1/600 of the wall height for walls with masonry veneer finishes.
21. Design framing systems to provide for movement of framing members without damage or overstressing sheathing failure, unless strain on fasteners and anchors, or other detrimental effects when subject to ambient temperature change of not less than 120 degrees.
22. Design framing system to maintain clearances at openings, to allow for construction tolerances, and to accommodate live load deflection of primary building structure as follows: Upward and downward movement of 3/4 inch.
23. Design exterior non-load-bearing curtain wall framing to accommodate horizontal deflection without regard for contribution of sheathing materials.

REINFORCED MASONRY NOTES

- 1. All construction of reinforced masonry walls to be in accordance with the Building Code Requirements for Concrete Masonry Structures (ACI 530) and Commentary.
A) m = 2000/Psi
B) Maximum height of masonry lift: 5'-0"
C) Maximum height of groud lift: 5'-0"
D) See Specifications for additional masonry wall information.
2. CONCRETE BLOCK: Minimum compressive test strength on the net cross-sectional area: 2800 PSI.
3. MORTAR: Type S required.
GROUT: ASTM C-796, 2000 PSI with a slump of 8" and a min. 117 max.
4. REINFORCING: #4 - 60000 PSI with a min. lap of 48 bar diameters.

CONTRACT NOTES

THE CONTRACT, BEHINDS, PLANS, DETAILS, ETC. SHOWN ON THIS DOCUMENT ARE THE PROPERTY OF GIBRALTAR DESIGN AND WILL BE RETURNED TO THE USER BY ANY PERSON OR FIRM WHOSE RESPONSIBILITY IT IS TO RETURN THESE DOCUMENTS TO GIBRALTAR DESIGN. THE OWNER MAY RETAIN COPIES FOR HIS OWN USE AND REFERENCE IN CONNECTION ONLY WITH THIS PROJECT.

REVISIONS

Table with columns: MARK, DATE, ISSUED FOR

DRAWING STRUCTURAL NOTES

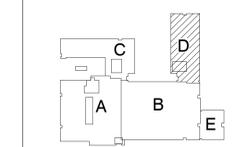
PROJECT EISENHOWER ELEMENTARY SCHOOL ADDITIONS, RENOVATIONS, AND RELATED WORK



GIBRALTAR
DESIGN
ARCHITECTURE · ENGINEERING · INTERIOR DESIGN

PROJECT
**EISENHOWER
ELEMENTARY
SCHOOL
ADDITIONS,
RENOVATIONS,
AND RELATED
WORK**

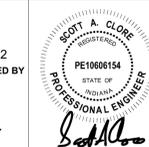
CROWN POINT COMMUNITY
SCHOOL CORPORATION
CROWN POINT, INDIANA



GIBRALTAR DESIGN

9102 N. Meridian St., Ste. 300
Indianapolis, IN 46260
Homepage: www.GibraltarDesign.com
Email: info@GibraltarDesign.com
Phone: 317.580.5777 Fax: 317.580.5778

PROJECT
21-116
DATE
08/18/2022
COORDINATED BY
NHF
DRAWN BY
NHF
CHECKED BY
SAC



THE CONTRACTOR, DESIGNER, PLANS, DETAILS, ETC. SHOWN ON THIS DOCUMENT ARE THE PROPERTY OF GIBRALTAR DESIGN AND ARE NOT TO BE REPRODUCED OR USED FOR ANY OTHER PROJECT WITHOUT THE EXPRESS WRITTEN CONSENT OF GIBRALTAR DESIGN. THE OWNER MAY RETAIN COPIES FOR INFORMATION AND REFERENCE IN CONNECTION ONLY WITH THIS PROJECT.

REVISIONS

MARK	DATE	ISSUED FOR
AD-1	08.31.2022	ADDENDUM NO. 1

DRAWING
FOUNDATION PLAN -
UNIT D

PROJECT
EISENHOWER ELEMENTARY
SCHOOL ADDITIONS,
RENOVATIONS,
AND RELATED WORK

SHEET
D S-201D

FOUNDATION PLAN KEYED NOTES

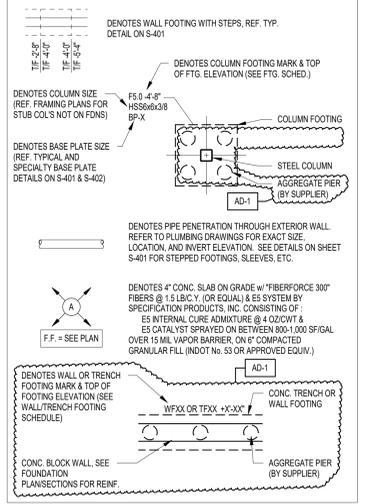
- APPROXIMATE LOCATION OF EXISTING PIPE TO BE REPLACED. COORDINATE WITH PLUMBING DRAWINGS AND FIELD VERIFY EXACT LOCATION AND DEPTH. PROVIDE STEPS IN FOOTING AND SLEEVES THROUGH FOUNDATION WALLS AS REQUIRED PER THE TYPICAL DETAILS ON SHEET S-401.
- IF PIPE INDICATED IN NOTE 1 IS LOCATED BELOW A NEW COLUMN FOOTING, AND THE DEPTH TO THE TOP OF THE PIPE IS LESS THAN 8 FEET BELOW THE BOTTOM OF THE COLUMN FOOTING, PROVIDE A "BRIDGE" DETAIL AS INDICATED IN SECTION 9S-402.

FOUNDATION PLAN NOTES

- REF. S-401 FOR STRUCTURAL NOTES, DESIGN DATA & SCHEDULES.
- ALL CONTRACTORS ARE REQUIRED TO COORDINATE THEIR WORK WITH ALL DISCIPLINES TO AVOID CONFLICTS. THE MECHANICAL, ELECTRICAL, AND PLUMBING ASPECTS ARE NOT IN THE SCOPE OF THESE DRAWINGS. THEREFORE, ALL REQUIRED MATERIALS AND WORK MAY NOT BE INDICATED.
- COORDINATE EXACT SIZE & LOCATION OF ALL MECHANICAL OPENINGS IN FOUNDATION WALLS WITH THE MECHANICAL, ELECTRICAL, & PLUMBING CONTRACTORS.
- ALL ELEVATIONS ARE REFERENCED FROM THE FIRST FLOOR FINISH FLOOR ELEVATION 100'-0" (TO MATCH EXISTING). REFER TO THE CIVIL DRAWINGS FOR EXACT U.S.G.S. ELEV.
- REF. ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION AND IMMEDIATELY NOTIFY ARCHITECT/ENGINEER OF ANY DISCREPANCIES. REF. S-401 FOR TYPICAL FOUNDATION DETAILS.
- NOTE: PERIMETER WALL AND COLUMN FOOTINGS SHALL BE LOWERED AND/OR SLEEVED TO PASS BELOW PLUMBING LINES (I.E. SANITARY & STORM SEWERS, WATER LINES, ETC.) SHOWN ON THE PLUMBING DRAWINGS. PROVIDE FOOTING STEPS AS REQUIRED PER THE TYPICAL DETAILS ON S-401.
- ALL SLAB RECESSES SHALL BE LOCATED PER THE ARCHITECTURAL DRAWINGS. COORDINATE DEPTHS OF ALL SLAB RECESSES WITH THE ARCHITECTURAL DRAWINGS AND/OR THE FLOORING SUPPLIER.
- COORDINATE REINFORCING DOWELS FOR CMU VERTICAL REINFORCING WITH REINF. NOTED ON PLANS & SECTIONS.
- GROUT ALL CORES OF CMU BELOW FINISH FLOOR SOLID.
- COLUMN FOOTINGS, TRENCH FOOTINGS AND WALL FOOTINGS SHALL BEAR ON APPROVED SOILS WITH A MINIMUM BEARING CAPACITY OF 2,500 PSF AT 12" D AND AT NEW (SCHED. FOOTING) OR EXISTING (EXIST. FOOTING) FOUNDATION.
- COLUMN FOOTINGS SUPPORTING MORE THAN ONE COLUMN SHALL BE CENTERED AT THE MIDPOINT BETWEEN THE COLUMNS, UNLESS NOTED OTHERWISE ON PLAN.
- PROVIDE THICKENED SLAB UNDER ALL INTERIOR WALLS WITHOUT FOOTINGS. SEE 4S-401 FOR THICKENED SLAB DETAIL. LAYOUT THICKENED SLABS FROM DIMENSIONS ON THE ARCHITECT FLOOR PLANS.
- PROVIDE CONTROL CONTRACTION JOINTS IN SLABS ON GRADE. REF. THE TYPICAL DETAILS ON SHEET S-401. ALL JOINTS IN SLABS TO RECEIVE THIN OR THICK-SET TERRAZZO, CERAMIC OR PORCELAIN TILE, VINYL COMPOSITION TILE (VCT) OR VINYL SHEET GOODS, EPOXY OR SIMILAR THIN FILM FINISH FLOORING SHALL BE CAREFULLY COORDINATED WITH THE FLOORING CONTRACTOR. THE CONTRACTOR SHALL SUBMIT SLAB JOINT LAYOUT TO ARCHITECT/ENGINEER FOR REVIEW PRIOR TO PLACING SLABS.
- FOR ARCHITECTURAL PLASTERS NOT SUPPORTING STEEL COLUMNS, CONSTRUCT AS FULLY-GROUTED MASONRY PIERS OR CAST-IN-PLACE CONCRETE PIERS REINFD W/ #5 VERTICAL REINFORCING AT 12" O.C. ALL FACES AT CONTRACTORS OPTION.
- LOWER BOTTOM OF FOOTINGS AS REQUIRED FOR PIPE SLEEVE. REFER TO DETAIL 11S-401. COORD. EXACT LOCATION AND INVERT ELEVATION w/ PLUMBING CONTRACTOR. STEP TOP OF FOOTING DOWN AS REQUIRED FOR PIPE SLEEVE. REFER TO DETAILS 10S-19S-401. COORD. EXACT LOCATION AND INVERT ELEVATION w/ PLUMBING CONTRACTOR.
- DENOTES INTERIOR CONC. BLOCK PARTITION WALL w/ #4 @ 48" o.c. (GROUT SOLID AT BARS) ON THICKENED SLAB PER DETAIL 4S-401.

PLAN LEGEND

F.F. DENOTES FINISH FLOOR
T/X DENOTES TOP OF FTG., GRADE BEAM, SLAB, PIER, ETC.
B/X DENOTES BOTTOM OF FTG., GRADE BEAM, ETC.
C.J. DENOTES SLAB ON GRADE CONTROL CONTRACTION JOINT
GB32x24-4'-0" DENOTES CONCRETE GRADE BEAM SIZE & TOP OF GRADE BEAM ELEVATION (SEE SCHEDULE)



COLUMN FOOTING SCHEDULE

- NOTES:
- CENTER FOOTINGS BENEATH COLUMNS, U.N.O.
 - ALL FOOTINGS MUST BE BOARD-FORMED, UNLESS APPROVED.
 - INCREASE FOOTING DEPTH WHERE REQ'D TO ENCASE COLUMN ANCHOR RODS.
 - IF STEEL COLUMN SHOWN, TUBE & PIPE COLUMNS SIM.

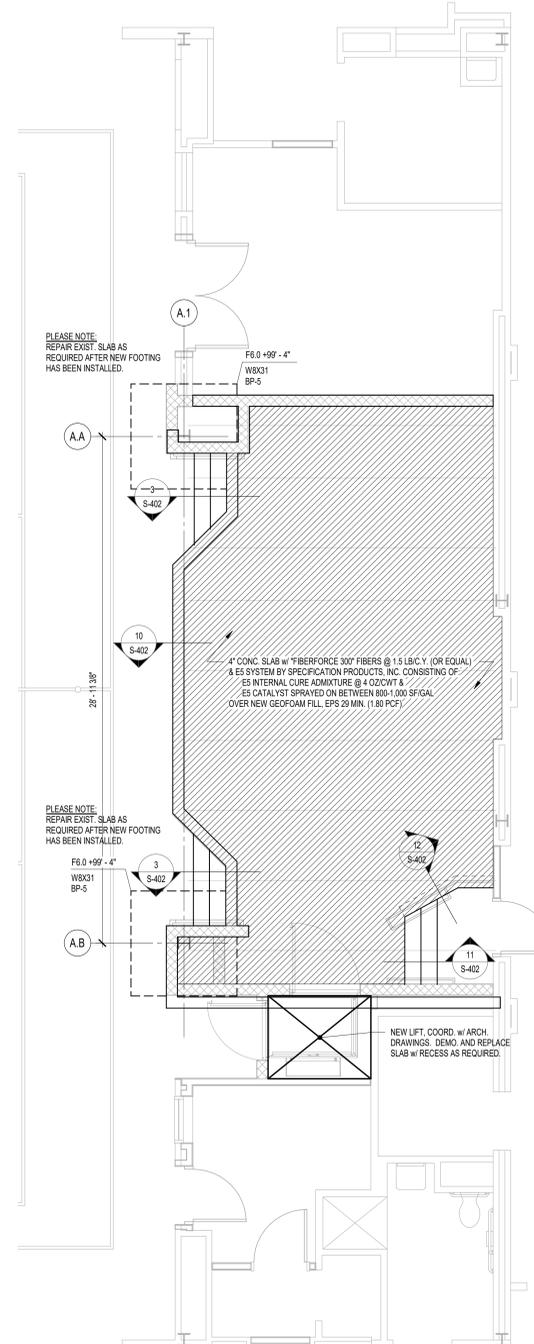
FOOTING MARK	LENGTH	WIDTH	DEPTH	REINFORCING
F5.0E	5'-0"	5'-0"	2'-4"	(6) #5 x 4'-6"
F6.0	6'-0"	6'-0"	1'-2"	(7) #5 x 5'-6"
F6.0E	6'-0"	6'-0"	2'-8"	(7) #5 x 5'-6"
F12.0x6.0	6'-0"	12'-0"	2'-4" (V.I.F.)	(13) #5 x 5'-6" (S.W.); (7) #5 x 1'-6" (U.W.)

TRENCH FOOTING SCHEDULE

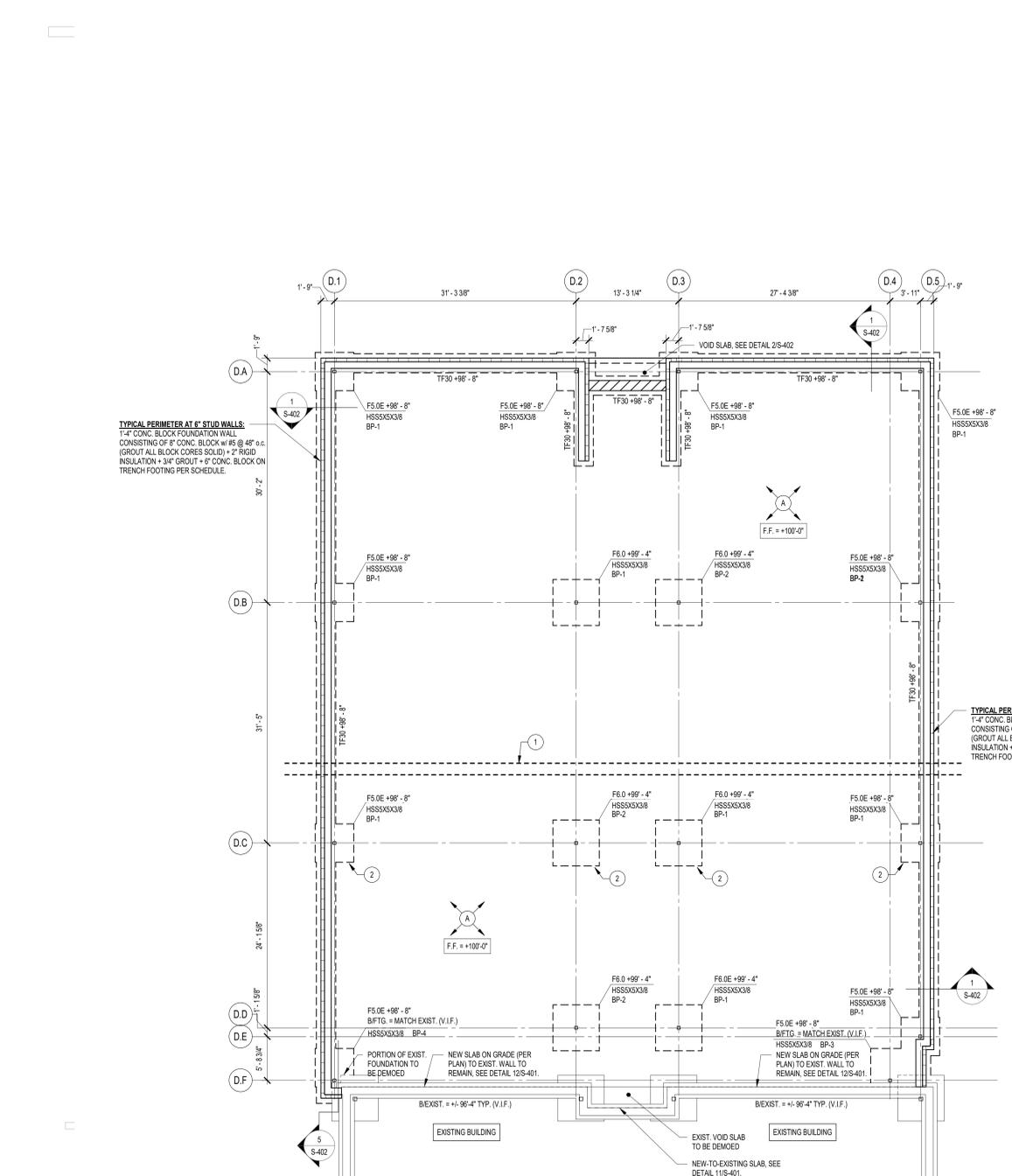
FTG. MARK	FOOTING SIZE		FOOTING REINFORCING	
	WIDTH	DEPTH	LONGITUDINAL	TRANSVERSE
TF30	2'-4"	2'-4"	(4) #5 x CONTINUOUS	#3 x 2'-0" @ 96" O.C.

1. CENTER FOOTINGS BENEATH COLUMNS, U.N.O.

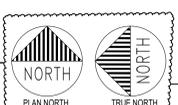
2. TRENCH FOOTINGS MAY BE CAST DIRECTLY AGAINST SOIL WITHOUT FORMING WHERE EXISTING SOIL CONDITIONS PERMIT. FORM TOP OF TRENCH FOOTINGS WHERE SOIL HAS SLOUGHED SIGNIFICANTLY, WHERE GRADE IS LOWER THAN THE INDICATED TOP OF FOOTING ELEVATION, OR WHEREVER TRENCH FOOTING WOULD INTERFERE WITH THE INSTALLATION OF DOWNSPOUTS, CONDUIT, BOLLARDS, ETC. COORDINATE WITH MECHANICAL, ELECTRICAL, PLUMBING & SITE/CIVIL DRAWINGS.



2 PARTIAL FOUNDATION PLAN - NEW STAGE
1/4" = 1'-0"



1 FOUNDATION PLAN - UNIT D
1/8" = 1'-0"





GIBRALTAR
DESIGN
ARCHITECTURE • ENGINEERING • INTERIOR DESIGN

PROJECT
**EISENHOWER
ELEMENTARY
SCHOOL
ADDITIONS,
RENOVATIONS,
AND
RELATED
WORK**
CROWN POINT COMMUNITY
SCHOOL CORPORATION
CROWN POINT, INDIANA

GIBRALTAR DESIGN
9102 N. Meridian St., Ste. 300
Indianapolis, IN 46260
Email: info@GibraltarDesign.com
Phone: 317.580.5777 Fax: 317.580.5778

PROJECT
DATE: 08/18/22
COORDINATED BY: CWT
DRAWN BY: PCO
CHECKED BY: CWT

COPYRIGHT NOTICE:
THE CONCEPTS, DESIGNS, PLANS, DETAILS, ETC. SHOWN ON THIS DOCUMENT ARE THE PROPERTY OF GIBRALTAR DESIGN AND ARE NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE EXPRESS WRITTEN CONSENT OF GIBRALTAR DESIGN. THE OWNER MAY RETAIN COPIES FOR INFORMATION AND REFERENCE IN CONNECTION ONLY WITH THIS PROJECT.

REVISIONS
MARK DATE ISSUED FOR
AD-1 08/31/22 ADDENDUM NO. 1

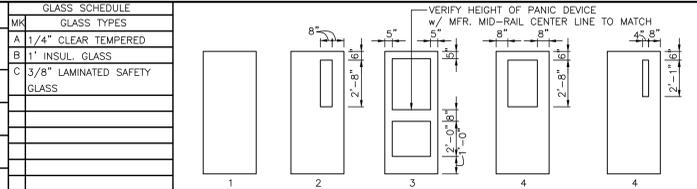
DRAWING
**DOOR SCHEDULE, FRAME
PROFILES, ELEVATIONS, AND
DETAILS**

PROJECT
**EISENHOWER ELEMENTARY
SCHOOL ADDITIONS, RENOVATIONS,
AND RELATED WORK**

© GIBRALTAR DESIGN SHEET
A-601

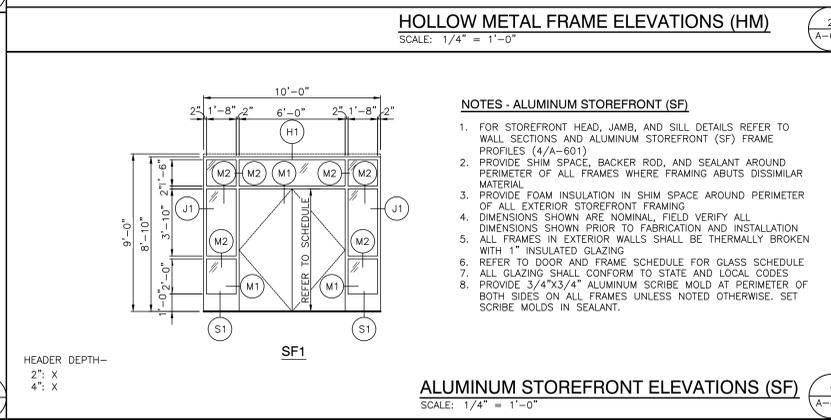
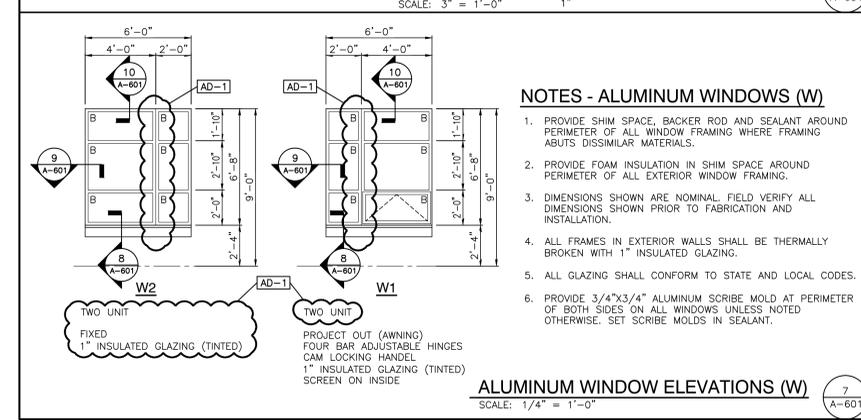
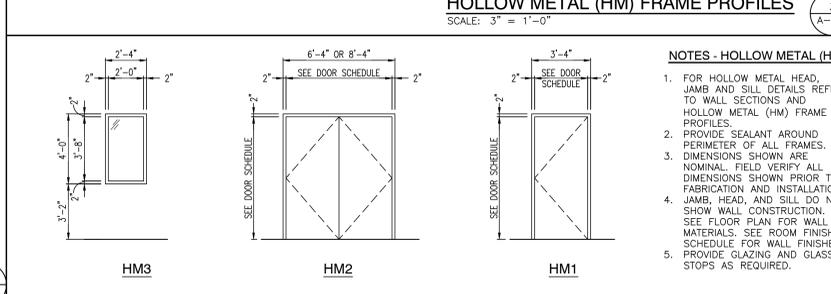
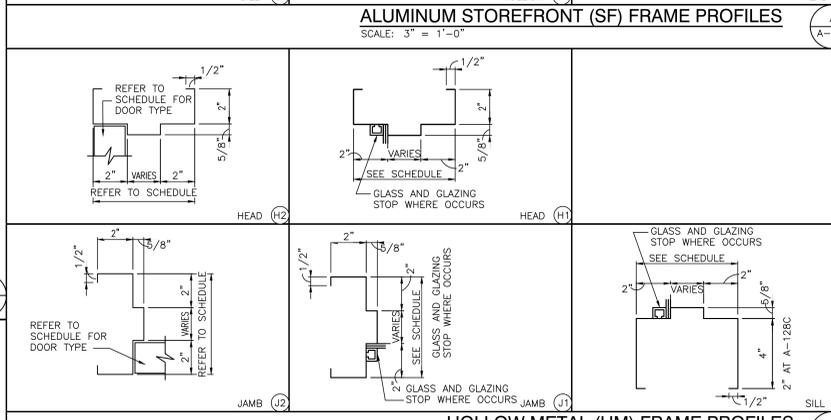
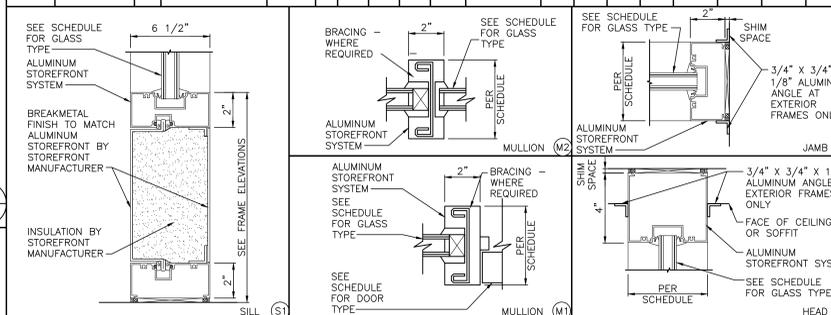
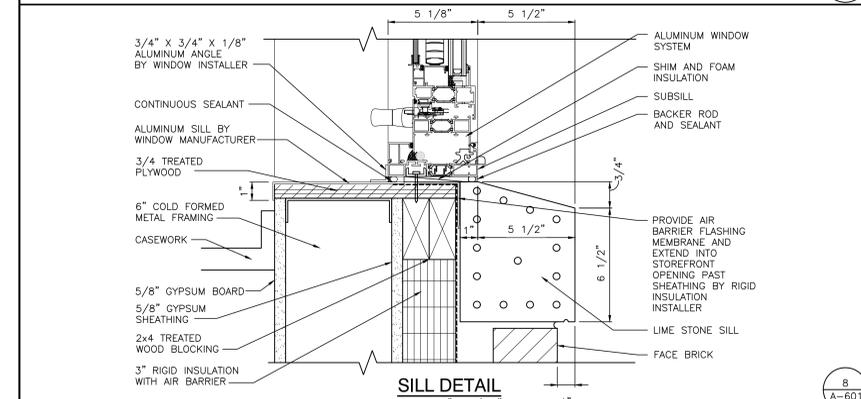
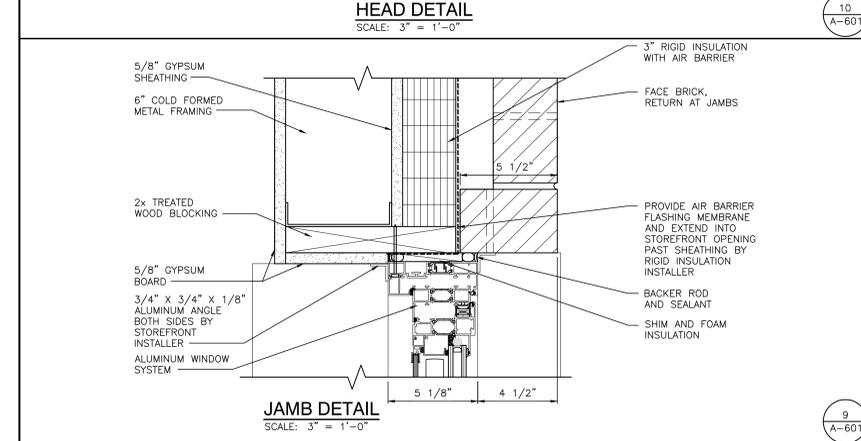
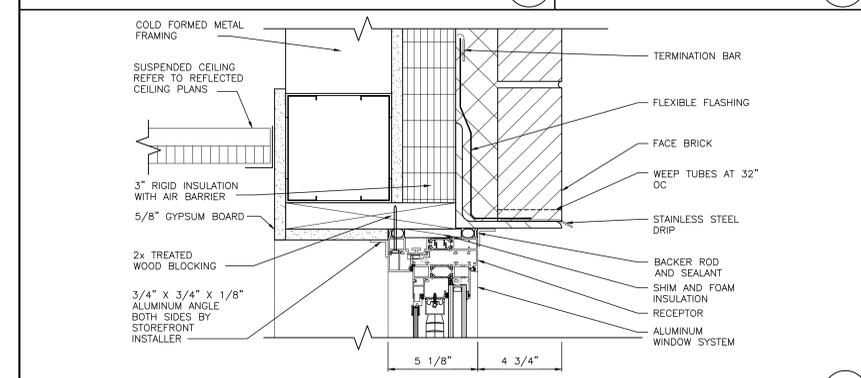
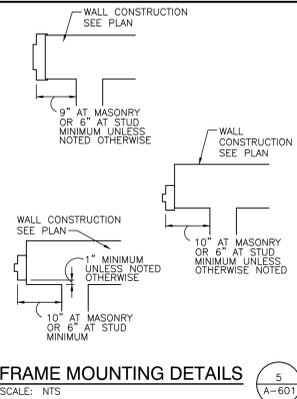
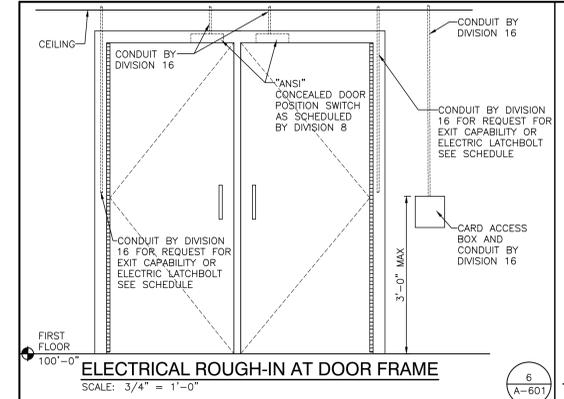
DOOR AND FRAME SCHEDULE

NO	DESCRIPTION	TYPE	DOOR SIZE (WxH) (INCHES)	MATERIAL	LOU	GLASS	TRA	MAT'L	WIDTH	JAMB	HEAD	SILL	ELEV	LABEL	HARDWARE	EXIT	CLOSER	NOTES
D-119B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
D-120A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
D-120B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
D-121A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
D-121B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
D-122A	DOUBLE	3	PR 36 x 84	AL		B	B	AL	6"	SEE ELEV	SEE ELEV	SEE ELEV	SF1		YES	YES	1,2,3,4,5	
D-122B	DOUBLE	3	PR 36 x 84	AL		A	A	AL	4 1/2"	SEE ELEV	SEE ELEV	SEE ELEV	SF1		DUMMY EXIT	YES		
E-102A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
E-103A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
E-104A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
E-104B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
E-105A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
E-105B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
E-106A	DOUBLE	3	PR 36 x 84	AL		B	B	AL	6"	SEE ELEV	SEE ELEV	SEE ELEV	SF1		YES	YES	1,2,3,4,5	
E-106B	DOUBLE	3	PR 36 x 84	AL		A	A	AL	4 1/2"	SEE ELEV	SEE ELEV	SEE ELEV	SF1		DUMMY EXIT	YES		



DOOR AND FRAME SCHEDULE

NO	DESCRIPTION	TYPE	DOOR SIZE (WxH) (INCHES)	MATERIAL	LOU	GLASS	TRA	MAT'L	WIDTH	JAMB	HEAD	SILL	ELEV	LABEL	HARDWARE	EXIT	CLOSER	NOTES
A-122A	EXISTING SINGLE TO REMAIN	2	36 x 84	WD				HM	9"									6
A-123A	EXISTING SINGLE TO REMAIN	1	36 x 84	WD				HM	6"									6
A-124A	SINGLE	1	36 x 84	WD				HM	7 1/8"	J2	H2		HM1					
A-125A	SINGLE	1	36 x 84	WD				HM	7 1/8"	J2	H2		HM1					
A-125B	SINGLE	1	36 x 84	WD				HM	6"	J2	H2		HM1					
A-126A	EXISTING SINGLE TO REMAIN	2	36 x 84	WD				HM	9"									6
A-128A	SINGLE	2	36 x 84	WD				HM	5 3/4"	J2	H2		HM1					10
A-128B	SINGLE	1	36 x 84	WD				HM	6"	J2	H2		HM1					10
A-128C	BORROWED LIGHT	1	28 x 48					HM	5 3/4"	J1	H1	S1	HM3					10
A-132A	SINGLE	1	36 x 84	WD				HM	5 3/4"	J2	H2		HM1					10
A-133A	DOUBLE	1	PR 36 x 84	WD				HM	5 3/4"	J2	H2		HM2					10
B-104D	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
B-106A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-111A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-119A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-132A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-134A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-149A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-150A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-152A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
B-158A	SINGLE	1	36 x 84	WD				HM	6 3/8"	J2	H2		HM1					
C-113A	EXISTING SINGLE RELOCATED	1	36 x 84	WD				HM	6 3/8"									7
C-114A	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
C-114B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
C-115A	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
C-116A	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
C-117A	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
C-118A	EXISTING SINGLE RELOCATED	1	36 x 84	WD				HM	6 3/8"									7
C-118B	EXISTING SINGLE RELOCATED	1	36 x 84	WD				HM	6 3/8"									7
D-102A	SINGLE	1	36 x 84	WD				HM	6 3/8"	J2	H2		HM2					
D-104A	SINGLE	1	36 x 82	WD				HM	7 1/8"	J2	H2		HM1					8,9
D-112A	SINGLE	1	36 x 84	WD				HM	6 3/8"	J2	H2		HM1					10
D-113A	SINGLE	1	36 x 84	WD				HM	6 3/8"	J2	H2		HM1					10
D-116A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
D-117A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
D-118A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					
D-118B	SINGLE	1	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					8,9
D-119A	SINGLE	2	36 x 84	WD				HM	8 3/8"	J2	H2		HM1					



NOTES - HOLLOW METAL (HM)

- FOR HOLLOW METAL HEAD, JAMB AND SILL DETAILS REFER TO WALL SECTIONS AND HOLLOW METAL (HM) FRAME PROFILES (4/A-601)
- PROVIDE SEALANT AROUND PERIMETER OF ALL FRAMES, DIMENSIONS SHOWN ARE NOMINAL. FIELD VERIFY ALL DIMENSIONS SHOWN PRIOR TO FABRICATION AND INSTALLATION.
- JAMB, HEAD, AND SILL DO NOT SHOW WALL CONSTRUCTION. SEE FLOOR PLAN FOR WALL MATERIALS. SEE ROOM FINISH SCHEDULE FOR WALL FINISHES.
- PROVIDE GLAZING AND GLASS STOPS AS REQUIRED.

NOTES - ALUMINUM STOREFRONT (SF)

- FOR STOREFRONT HEAD, JAMB, AND SILL DETAILS REFER TO WALL SECTIONS AND ALUMINUM STOREFRONT (SF) FRAME PROFILES (4/A-601)
- PROVIDE SHIM SPACE, BACKER ROD, AND SEALANT AROUND PERIMETER OF ALL FRAMES WHERE FRAMING ABUTS DISSIMILAR MATERIAL.
- PROVIDE FOAM INSULATION IN SHIM SPACE AROUND PERIMETER OF ALL EXTERIOR STOREFRONT FRAMING.
- DIMENSIONS SHOWN ARE NOMINAL. FIELD VERIFY ALL DIMENSIONS SHOWN PRIOR TO FABRICATION AND INSTALLATION.
- ALL FRAMES IN EXTERIOR WALLS SHALL BE THERMALLY BROKEN WITH 1\"/>

GENERAL DOOR NOTES:

- JAMB, HEAD, AND SILL DO NOT SHOW WALL CONSTRUCTION. SEE FLOOR PLAN FOR WALL MATERIALS. REFER TO A800 SERIES DRAWINGS FOR WALL FINISHES.
- SEAL ALL JAMBS AND HEADS WHERE FRAMES MEET EXPOSED MASONRY AND/OR GYPSUM BOARD.
- PROVIDE A SCRIBE MOLD AT ALL EXTERIOR DOOR FRAMES AND WHERE NOTED ON DRAWINGS. SCRIBE MOLD TO BE 3/4\"/>

DOOR SCHEDULE NOTES (REMARKS):

- PANIC DEVICE TO HAVE ELECTRIC LATCH BOLT. PREPARE FRAME FOR ELECTRIFIED HINGE.
- DOOR TO BE CONTROLLED BY CARD READER. REFER TO ELECTRICAL DRAWINGS FOR ROUGH IN.
- PROVIDE ALUMINUM THRESHOLD SET IN FULL BED OF MASTIC. REFER TO 3/A-610.
- KEYED REMOVABLE MULLION.
- PREPARE DOOR AND FRAME FOR DOOR POSITION SWITCH.
- EXISTING METAL FRAME, DOOR, AND HARDWARE TO REMAIN.
- INSTALL SALVAGED METAL FRAME, DOOR, AND HARDWARE.
- PROVIDE DROP SEAL, SOUND GASKETING AND ADA COMPLIANT THRESHOLD.
- DOORS TO BE LOCKABLE FROM BOTH SIDE.
- ALTERNATE.

NOTES - ALUMINUM WINDOWS (W)

- PROVIDE SHIM SPACE, BACKER ROD AND SEALANT AROUND PERIMETER OF ALL WINDOW FRAMING WHERE FRAMING ABUTS DISSIMILAR MATERIALS.
- PROVIDE FOAM INSULATION IN SHIM SPACE AROUND PERIMETER OF ALL EXTERIOR WINDOW FRAMING.
- DIMENSIONS SHOWN ARE NOMINAL. FIELD VERIFY ALL DIMENSIONS SHOWN PRIOR TO FABRICATION AND INSTALLATION.
- ALL FRAMES IN EXTERIOR WALLS SHALL BE THERMALLY BROKEN WITH 1\"/>

NOTES - ALUMINUM STOREFRONT (SF)

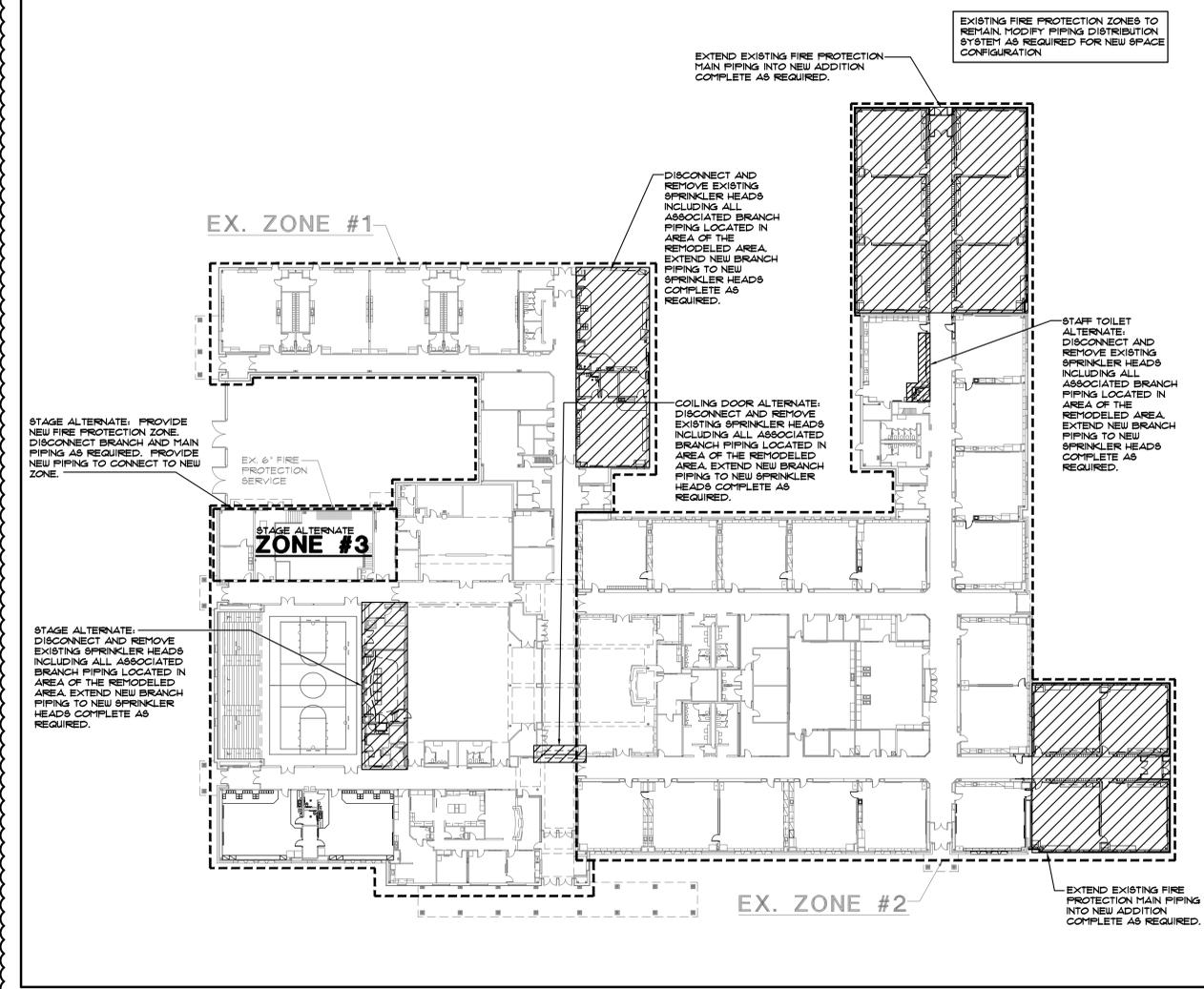
- FOR STOREFRONT HEAD, JAMB, AND SILL DETAILS REFER TO WALL SECTIONS AND ALUMINUM STOREFRONT (SF) FRAME PROFILES (4/A-601)
- PROVIDE SHIM SPACE, BACKER ROD, AND SEALANT AROUND PERIMETER OF ALL FRAMES WHERE FRAMING ABUTS DISSIMILAR MATERIAL.
- PROVIDE FOAM INSULATION IN SHIM SPACE AROUND PERIMETER OF ALL EXTERIOR STOREFRONT FRAMING.
- DIMENSIONS SHOWN ARE NOMINAL. FIELD VERIFY ALL DIMENSIONS SHOWN PRIOR TO FABRICATION AND INSTALLATION.
- ALL FRAMES IN EXTERIOR WALLS SHALL BE THERMALLY BROKEN WITH 1\"/>

Monday, 8/29/2022 - 11:04 AM - LAST SAVED BY: PEGRETHOY
Y:\21-116 CROWN POINT CSC - EISENHOWER ES ADDITION AND RENOVATIONS\21-116 DRAWINGS\05 ARCH-A-601.DWG

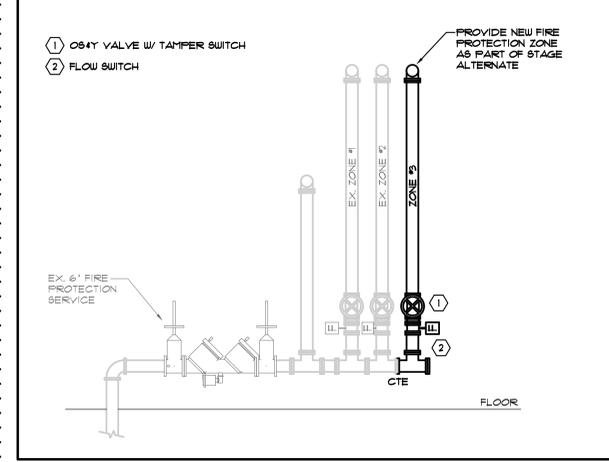
GENERAL NOTES

- A. WORK SHALL COMPLY WITH LOCAL, MUNICIPAL, STATE FIRE PROTECTION CODES, THE LATEST NFPA 13 REQUIREMENTS.
- B. THE SCOPE OF WORK SPECIFIED HEREIN AND IN THE SPECIFICATIONS SHALL BE COORDINATED WITH THE CONSTRUCTION MANAGER - REFER TO THE SCOPE OF WORK FOR EACH TRADE. ANY DISCREPANCIES BETWEEN THE CONSTRUCTION DOCUMENTS AND CONSTRUCTION MANAGER'S SCOPE SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER FOR CLARIFICATION. THE ARCHITECT/ENGINEER'S DECISION SHALL BE FINAL.
- C. THE ENTIRE BUILDING OR THE NEW ADDITIONS SHALL BE FURNISHED AND INSTALLED WITH A COMPLETE AUTOMATIC SPRINKLER SYSTEM. EXISTING, REMODELED AND NEW ADDITION AREAS OF THIS BUILDING SHALL BE COVERED INCLUDING TUNNEL, ATTIC, CRAWLSPACE AND INTERSTITIAL SPACES.
- D. CONNECT EXISTING LOCAL AND EXISTING MAIN FIRE PROTECTION SERVICES TO NEW SERVICE AS REQUIRED. PROVIDE NECESSARY ZONING AND FIRE PROTECTION MAINS TO EXISTING LOCATIONS AS REQUIRED.
- E. THE SPRINKLER SYSTEM SHALL BE DESIGNED TO DELIVER A DENSITY OF 1.0 GPM OVER THE MOST REMOTE 1500 SQFT, WITH ALLOWANCE FOR 100 GPM OUTSIDE HOSE STREAM. CALCULATIONS SHALL BE IN ACCORDANCE WITH THE LATEST NFPA 13 CHAPTER FOR LIGHT HAZARD.
- F. THE BUILDING HAS MULTIPLE OCCUPANCY CLASSIFICATIONS AND THE SPRINKLER SYSTEM SHALL BE DESIGNED FOR EACH CLASSIFICATION ACCORDING TO NFPA REQUIREMENTS. FOR PORTIONS OF THE FACILITY WITH AREAS OF DIFFERENT CLASSIFICATIONS THAT ARE NOT PHYSICALLY SEPARATED BY A BARRIER OR PARTITION, THE REQUIRED SPRINKLER PROTECTION FOR THE MORE DEMANDING AREA SHALL EXTEND 15'-0" BEYOND ITS PERIMETER.
- 1. GENERAL AREAS ARE LIGHT HAZARD, 0.10 GPM/SQFT. FOR THE MOST REMOTE 1500 SQFT.
- 2. KITCHEN AREA IS ORDINARY HAZARD (GROUP 1), 0.15 GPM/SQFT. FOR THE MOST REMOTE 1500 SQFT.
- 3. STAGE AREA IS ORDINARY HAZARD (GROUP 2), 0.20 GPM/SQFT. FOR THE MOST REMOTE 1500 SQFT.
- G. THE REMODELED AREA SHALL HAVE THE EXISTING SPRINKLER SYSTEM MODIFIED WITH NEW SPRINKLER HEADS DESIGNED TO DELIVER A DENSITY OF 1.0 GPM OVER THE MOST REMOTE 1500 SQFT. CALCULATIONS SHALL BE IN ACCORDANCE WITH THE LATEST NFPA 13 CHAPTER FOR LIGHT HAZARD.
- H. LAYOUT IS DIAGRAMMATIC. INSTALL PIPING AND EQUIPMENT TO MEET ACTUAL FIELD CONDITIONS. REVIEW PROJECT SPECIFICATIONS BEFORE STARTING ANY WORK. SUBMIT SHOP DRAWINGS OF WORK AS PER SPECIFICATIONS.
- I. VERIFY IF EXISTING ASBESTOS WILL BE ENCOUNTERED PRIOR TO STARTING ANY WORK. IF ASBESTOS IS PRESENT, THE OWNER WILL PROVIDE FOR THE REMOVAL OF ANY MATERIAL CONTAINING ASBESTOS. SEE SPECIFICATIONS FOR FURTHER REQUIREMENTS.
- J. COORDINATE PHASING OF WORK AND PROVIDE TEMPORARY PIPING AND SERVICES AS REQUIRED FOR THE IMPLEMENTATION OF WORK WHILE MAINTAINING SERVICES TO PORTIONS OF BUILDING TO REMAIN OCCUPIED.
- K. SCHEDULE WORK TO AVOID DOWNTIME AND INCONVENIENCE TO OWNER. OWNER'S EXISTING FACILITY SHALL REMAIN IN OPERATION AT TIMES. REQUIRED SHUTDOWNS OF EXISTING UTILITIES SHALL BE SCHEDULED WITH OWNER'S OPERATING PERSONNEL. NOTIFY OWNER'S REPRESENTATIVE 48 HOURS IN ADVANCE PRIOR TO ANY SHUTDOWNS OF EXISTING SYSTEMS.
- L. FIRE PROTECTION PIPING ROUTING TO BE FIELD COORDINATED WITH NEW AND EXISTING HVAC DUCTWORK, HVAC PIPING, PLUMBING PIPING AND STRUCTURE TO ENSURE NO CONFLICTS WILL OCCUR DUE TO INTERFERENCE.
- M. REFER TO ARCHITECTURAL REFLECTED CEILING PLANS FOR HVAC DIFFUSER LAYOUT AND ELECTRICAL SPECIALTY DEVICES IN CONJUNCTION WITH ELECTRICAL LIGHTING, SPRINKLER HEAD LAYOUT, AND CEILING GRID SYSTEM.
- N. VISIT SITE PRIOR TO BIDDING TO DETERMINE FIELD CONDITIONS. COORDINATE NEW INSTALLATIONS WITH EXISTING SYSTEMS, EXISTING CONDUIT, PIPING, DUCTWORK, EQUIPMENT, ETC. SHALL BE REWORKED AS REQUIRED TO AVOID CONFLICTS WITH THE INSTALLATION OF THE NEW FIRE PROTECTION SYSTEMS. NO EXTRAS WILL BE ALLOWED AFTER BIDDING FOR ANY REWORK OF EXISTING FIELD CONDITIONS TO RESOLVE CONFLICTS OR NOT FULLY UNDERSTANDING THE SCOPE OF THE WORK. REQUIRED.
- O. EXISTING INFORMATION IDENTIFIED ON THE CONTRACT DOCUMENTS IS SCHEMATIC ONLY AS AN AID TO THE CONTRACTOR. PROVIDE WRITTEN ADDRESS EXISTING CONDITIONS FOR A COMPLETE AND PROPER INSTALLATION OF NEW SYSTEMS. EXISTING EQUIPMENT NOT IDENTIFIED SHALL BE REPORTED IN WRITTEN FORM FOR REVIEW AS TO WHETHER THE EQUIPMENT SHALL REMAIN AND BE RECONNECTED TO THE NEW SERVICES, BE RELOCATED, BE ABANDONED, ETC.
- P. HIDDEN CONDITIONS IDENTIFIED THROUGH THE COURSE OF CONSTRUCTION SHALL BE IMMEDIATELY REPORTED IN WRITTEN FORM FOR REVIEW AND DIRECTION. FAILURE TO DO SO SHALL MAKE THE CONTRACTOR RESPONSIBLE FOR REQUIRED CHANGES AND COSTS TO CORRECT SAID HIDDEN CONDITION.
- Q. COORDINATE NEW INSTALLATIONS WITH EXISTING SYSTEMS, EXISTING CONDUIT, PIPING, DUCTWORK, EQUIPMENT, ETC. SHALL BE REWORKED AS REQUIRED TO AVOID CONFLICTS WITH THE INSTALLATION OF THE NEW FIRE PROTECTION SYSTEMS. NO EXTRAS WILL BE ALLOWED AFTER BIDDING FOR ANY REWORK OF EXISTING FIELD CONDITIONS TO RESOLVE CONFLICTS OR NOT FULLY UNDERSTANDING THE SCOPE OF THE WORK. REQUIRED.
- R. PROVIDE CUTTING, CORE DRILLING AND PATCHING OF EXISTING FLOOR AND WALL CONSTRUCTIONS REQUIRED FOR THE INSTALLATION OF NEW PIPING. SEAL PENETRATIONS THROUGH FLOOR, WALL, AND ROOF STRUCTURE WATER TIGHT AND WITH AN APPROVED FIRE STOPPING MATERIAL INCLUDING APPROVED FIRE RATED SLEEVE.
- S. CUT OR CHANNEL INTO EXISTING WALL CONSTRUCTIONS AS REQUIRED FOR INSTALLATION OF NEW PIPING WITHIN EXISTING WALLS. PATCH WALL SURFACES AND FINISH AS REQUIRED TO MATCH EXISTING CONDITIONS.
- T. REMOVE EXISTING CEILING REQUIRED FOR INSTALLATION OF NEW WORK. REINSTALL CEILING UPON COMPLETION OF WORK - REPLACE DAMAGED CEILING MATERIALS TO MATCH EXISTING. GYPSUM BOARD CEILING: PROVIDE CONCEALED CONTROL JOINT AT EDGES ABUTTING EXISTING GYPSUM BOARD CEILING. TAPE IN NEW AREAS TO EXISTING FLUSH - PROVIDE TEXTURE TO MATCH EXISTING.
- U. EXISTING EQUIPMENT SHALL REMAIN PROPERTY OF THE OWNER AND OWNER SHALL DETERMINE IF EQUIPMENT IS TO BE STORED ON SITE AT OWNER SELECTED LOCATION OR IF IT IS TO BE ABANDONED OR REMOVED FROM SITE.
- V. REMOVE EXISTING SPRINKLER HEADS AND ASSOCIATED BRANCH SPRINKLER PIPING COMPLETE AS REQUIRED. EXTEND AND MODIFY EXISTING PIPING AS REQUIRED FOR NEW SPRINKLER HEAD LAYOUT.
- W. REMOVED PIPING IS TO BE TERMINATED PROPERLY BACK TO EXISTING MAINS. CAP PIPING WATER TIGHT. PROVIDE ADDITIONAL PIPING AS REQUIRED TO MAINTAIN CONTINUITY OF EXISTING SYSTEMS MODIFIED DUE TO REMOVAL OF PORTION OF SYSTEMS. NO EQUIPMENT, PIPING, SUPPORTS, HANGERS, ETC. IS TO BE LEFT ABANDONED. VERIFY QUANTITY, LOCATION AND ELEVATION OF EXISTING TO BE REMOVED IN FIELD.
- X. PATCH EXISTING CEILING, FLOOR, WALL AND ROOF OPENINGS AND SURROUNDING FINISHES RESULTING FROM REMOVAL OF EXISTING MATERIALS AND EQUIPMENT SO THAT FINISH WILL MATCH EXISTING IN SURROUNDING AREAS.
- Y. PROVIDE FINISHING OF EXISTING CEILING, FLOOR, AND WALL SURFACES AT LOCATIONS EFFECTED BY REMOVAL OF EXISTING MATERIALS AND EQUIPMENT SO THAT NEW FINISH WILL MATCH EXISTING IN SURROUNDING AREAS.
- Z. REMOVE EXISTING CEILING AND LIGHT FIXTURES REQUIRED FOR INSTALLATION OF NEW WORK. REINSTALL CEILING AND LIGHT FIXTURES UPON COMPLETION OF WORK. REPLACE DAMAGED CEILING MATERIALS TO MATCH EXISTING.
- AA. CUT OR CHANNEL INTO EXISTING WALL CONSTRUCTIONS AS REQUIRED FOR INSTALLATION OF NEW PIPING WITHIN EXISTING WALLS. PATCH WALL SURFACES AND FINISH AS REQUIRED TO MATCH EXISTING CONDITIONS.
- BB. SPRINKLER HEADS IN FINISHED CEILING SPACES SHALL BE SEMI-RECESSED TYPE WITH WHITE ESCUTCHEON PLATE OR CONCEALED TYPE. SPRINKLER HEADS IN UNFINISHED SPACES SHALL BE PENDENT, UPRIGHT, OR SIDEWALL TYPE.
- CC. SPRINKLER HEADS LOCATED IN SUSPENDED LAY-IN CEILING SYSTEMS SHALL BE CENTERED IN EACH RESPECTIVE TILE. PROVIDE 6" SWINGS TO PLACE THE SPRINKLER IN THE CENTER OF THE TILE.
- DD. PIPING SHALL BE HYDROSTATICALLY TESTED AT 200 PSI OR 50 PSI OVER THE MAXIMUM OPERATING PRESSURE, WHICHEVER IS GREATER, FOR A PERIOD OF TWO HOURS.
- EE. PIPES SHALL BE SUPPORTED FROM THE BUILDING STRUCTURE IN ACCORDANCE WITH THE LATEST NFPA 13.
- FF. PIPING, EQUIPMENT, ETC. SHALL NOT BE SUPPORTED FROM THE BOTTOM CHORD OF ENGINEERED JOISTS WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.
- GG. WET SYSTEM PIPING SHALL BE INSTALLED LEVEL TO DRAIN BACK TO THE SYSTEM RISER. TRAPPED SECTIONS OF PIPING SHALL HAVE AUXILIARY DRAIN CONNECTIONS IN ACCORDANCE WITH LATEST NFPA 13.
- HH. PROVIDE EXTENDED COVERAGE HEADS IN CORRIDORS WIDER THAN 15'-0".
- II. PROVIDE A WATER TIGHT SHEET METAL DRIP PAN OVER ELECTRICAL EQUIPMENT INSTALLED UNDER OR NEAR PIPING SYSTEMS. DRIP PAN TO EXTEND MINIMUM 3" OVER FRONT AND SIDES OF ELECTRICAL EQUIPMENT AND BE PITCHED AT A MINIMUM 30° ANGLE. SEAL DRIP PAN WATER TIGHT TO WALL.
- JJ. PIPING PENETRATING AIR PLENUM CEILING AREAS SHALL BE PROPERLY SEALED TO MAKE AIR TIGHT. REFER TO MECHANICAL DUCTWORK DRAWINGS FOR AIR PLENUM LOCATIONS.
- KK. PROVIDE 1" INSPECTOR'S TEST CONNECTIONS FOR EACH ZONE TO ALLOW THE FLOW TESTING OF THE WATER FLOW INDICATOR SWITCH IN THE SPRINKLER RISER. THE TEST CONNECTION SHALL HAVE A 1" GLOBE VALVE LOCATED AT 1'-0" ABOVE THE FLOOR AND SHALL BE ARRANGED TO DISCHARGE THROUGH A 1/2" SMOOTH BORE BRASS BUSHIN.

FIRE PROTECTION ZONING PLAN



FIRE PROTECTION SERVICE DIAGRAM



GIBRALTAR DESIGN
ARCHITECTURE • ENGINEERING • INTERIOR DESIGN

MILLIES ENGINEERING GROUP
(219) 924-8400
www.milliesengineeringgroup.com

PROJECT
EISENHOWER ELEMENTARY SCHOOL ADDITIONS, RENOVATIONS, AND RELATED WORK
CROWN POINT COMMUNITY SCHOOL CORPORATION
CROWN POINT, INDIANA

GIBRALTAR DESIGN
9102 N. Meridian St., Ste. 300
Indianapolis, IN 46260
Homepage: www.GibraltarDesign.com
Email: info@GibraltarDesign.com
Phone: 317.580.5777 Fax: 317.580.5778

PROJECT
21-116
DATE
08/18/22
COORDINATED BY
SM
DRAWN BY
CC
CHECKED BY
DJ

COPYRIGHT NOTICE:
THE CONCEPTS, DESIGNS, PLANS, DETAILS, ETC. SHOWN ON THIS DOCUMENT ARE THE PROPERTY OF GIBRALTAR DESIGN AND WERE CREATED FOR USE ON THIS SPECIFIC PROJECT. NONE OF THIS INFORMATION SHALL BE USED BY ANY PERSON OR FIRM FOR ANY PURPOSE WITHOUT THE EXPRESS WRITTEN CONSENT OF GIBRALTAR DESIGN. THE OWNER MAY RETAIN COPIES FOR INFORMATION AND REFERENCE IN CONNECTION ONLY WITH THIS PROJECT.

REVISIONS	MARK	DATE	ISSUED FOR
AD-1	08/31/22	ADDENDUM NO. 1	

DRAWING
FIRE PROTECTION PLAN, NOTES DETAILS & DIAGRAMS

PROJECT
EISENHOWER ELEMENTARY SCHOOL ADDITIONS, RENOVATIONS, AND RELATED WORK

© GIBRALTAR DESIGN SHEET
FP001